

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Habitat for Humanity of Colorado**.

These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: **PP - 3410\_0 - A Risley4Bdr1S - 1122757**  
L01, L02, L03, L04, T01, T02, T03, T04, T05, V01, V02, V03, V04, V05

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.



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Arturo A. Hernandez (CO, PE-39632)  
My license renewal date for the state of CO is 10/31/2021.

**IMPORTANT NOTE:** The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

## DESIGN NOTES

1. The Truss Design Drawing(s) provided with these General Notes have been prepared under and are subject to ANSI/TPI1. Capitalized terms have the meanings provided in ANSI/TPI1.
2. Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.

### DESIGN LIMITATIONS

3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANSI/TPI1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
6. Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

### HANDLING, INSTALLING, & BRACING

7. Refer to BCSI for handling, installing, restraining and bracing trusses. Copies can be obtained from the Truss Plate Institute (TPI), 218 N Lee Street, Suite 312, Alexandria, VA 22314, www.tpinst.org or SBCA, 6300 Enterprise Lane, Madison, WI 53719, www.sbcindustry.com.
8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
9. Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
10. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.

11. Bottom chord required bracing shall be at 10ft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
12. Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
13. Never exceed the design loading shown and never stack building or other materials on inadequately braced truss; refer to BCSI.
14. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
15. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

### MATERIALS & FABRICATION

16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
17. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
18. Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI/TPI1.
20. For a specified plate gauge and grade, the specified size is a minimum.
21. Connections not shown are the responsibility of others.
22. Adequate support shall be provided to resist gravity, lateral, uplift loads.
23. For 4X2 truss orientation, locate plates 0 - 1/16" from outside the edge of the truss.
24. Fabrication of truss shall be in accordance with ANSI/TPI1.

### OTHER NOTES

25. Camber is a non-structural consideration and is the responsibility of truss fabricator.
26. Do not cut or alter any truss member or plate without prior approval from a professional engineer.
27. Lumber design values are in accordance with ANSI/TPI; lumber design values are by others.
28. Install specified hangers per manufacturer recommendations.

## SYMBOLS

### PLATE SIZE

**3X4** - The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

-, /, |, Indicates required direction of slots; Reference "Joint Details" for more information.

20 Ga Gr40 connectors required  
**3X10-20HS** - 20 Ga Gr60 connectors required  
**8X10-18HS** - 18 Ga Gr60 connectors required

### LATERAL BRACING

When this symbol shown, continuous lateral bracing is required on the web of the truss.



### BEARING

Indicates location where bearings (supports) occur.



### PLATE LOCATION & ORIENTATION

The plate shall be centered on joint and/or placed in accordance with the design drawing/QC full scale details.



## REFERENCES

- ANSI/TPI1:** National Design Standard for Metal Plate Connected Wood Trusses
- BCSI:** Building Component & Safety Information - Guide to Good Practice for Handling, Installing, Restraining, & Bracing of Metal Plate Connected Wood Trusses.
- NDS:** National Design Specification for Wood Construction
- ESR:** 1082 published by the International Code Council. www.icc-es.org

### Habitat For Humanity

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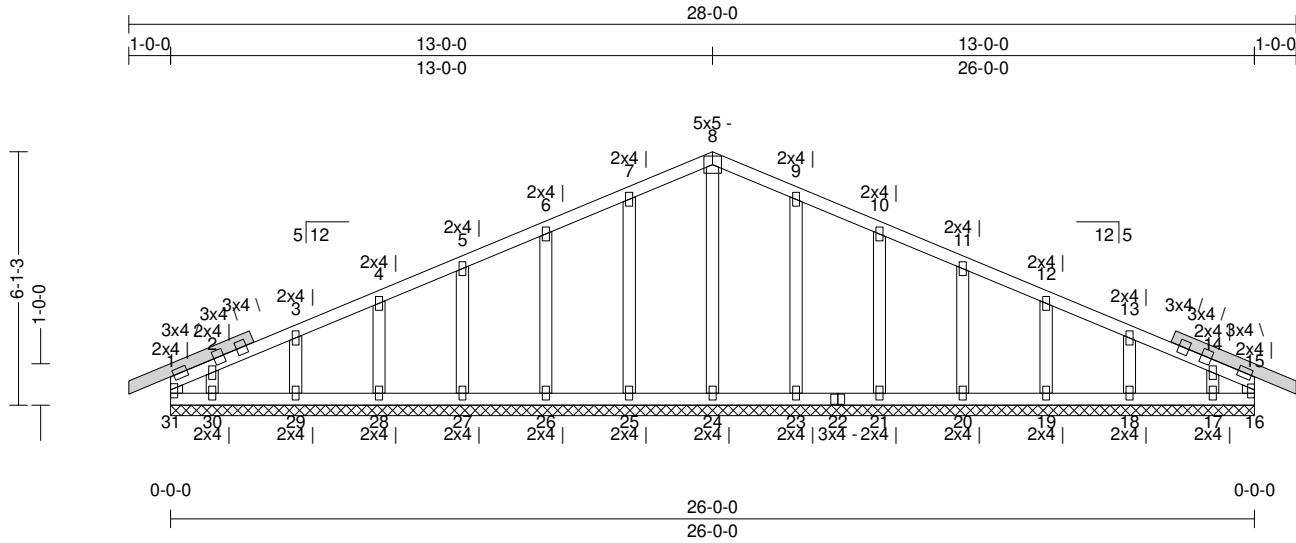
Truss: L01

Job: PP - 3410\_0 - ARisley4Bdr1S

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SPAN 26-0-0	PITCH 5/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 112 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IRC 2015/	TC: 0.21 (1-2)	Vert TL: 0 in UP	L/999	16	L/240
TCDL: 15	TP1 1-2014	BC: 0.02 (17-18)	Vert LL: 0 in	L/999	16	L/360
BCLL: 0	Rep Mbr: No	Web: 0.10 (1-31)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

03/17/2020

#### Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		306 lbs	139 plf	-5 lbs	-65 lbs	-86 lbs	-86 lbs	-121 lbs

#### Material

TC: SPF 1650/1.5 2 x 4  
BC: SPF 1650/1.5 2 x 4  
Web: SPF 1650/1.5 2 x 4

#### Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

#### Loads

- 1) This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL = 1.15.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

#### Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	
BC	
Web	

#### Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 24" OC, U.N.O.
- 4) Attach gable webs with 2x4 20ga plates, U.N.O.
- 5) Stitch top chords together with 20Ga plates at 24 in oc maximum, U.N.O.
- 6) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 7) The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- 8) A creep factor of 1.00 has been applied for this truss analysis.
- 9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 17, 30 may need to be considered.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.



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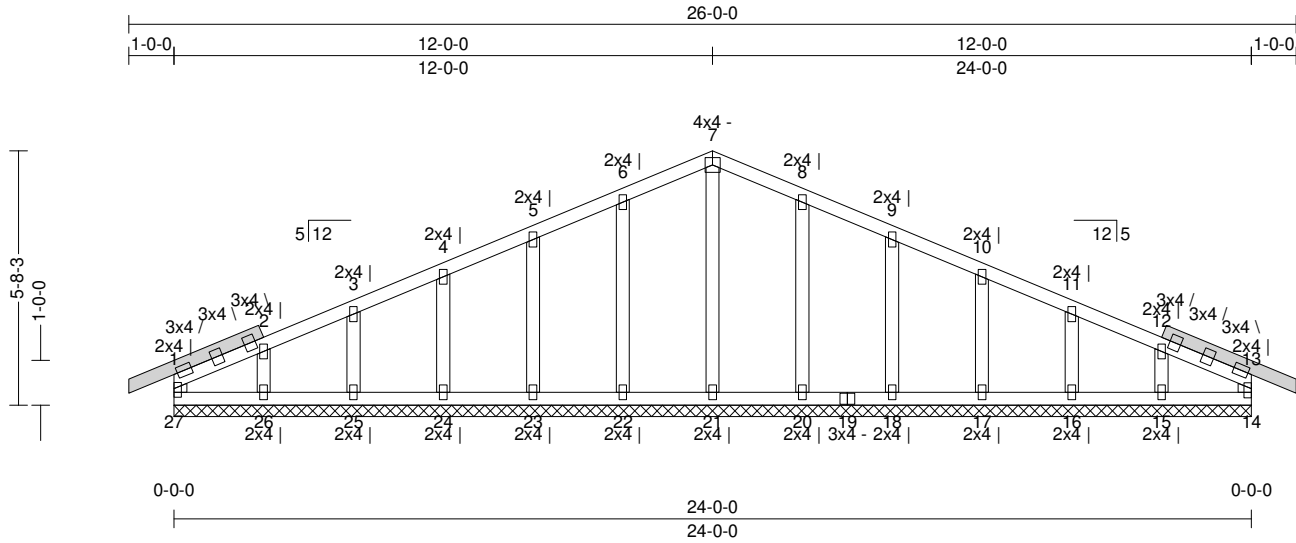
Truss: L02

Job: PP - 3410\_0 - ARisley4Bdr1S

Date: 03/17/20 07:44:40

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SPAN 24-0-0	PITCH 5/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 101 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IRC 2015/	TC: 0.21 (1-2)	Vert TL: 0 in UP	L/999	14	L/240
TCDL: 15	TP1 1-2014	BC: 0.02 (14-15)	Vert LL: 0 in	L/999	14	L/360
BCLL: 0	Rep Mbr: No	Web: 0.10 (1-27)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

03/17/2020

**Reaction**

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		305 lbs	138 plf		-65 lbs	-107 lbs	-107 lbs	-109 lbs

**Material**

TC: SPF 1650/1.5 2 x 4  
BC: SPF 1650/1.5 2 x 4  
Web: SPF 1650/1.5 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- 1) This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/CtCs = 1, DOL = 1.15.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

**Member Forces**

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 24" OC, U.N.O.
- 4) Attach gable webs with 2x4 20ga plates, U.N.O.
- 5) Stitch top chords together with 20Ga plates at 24 in oc maximum, U.N.O.
- 6) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 7) The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- 8) A creep factor of 1.00 has been applied for this truss analysis.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.



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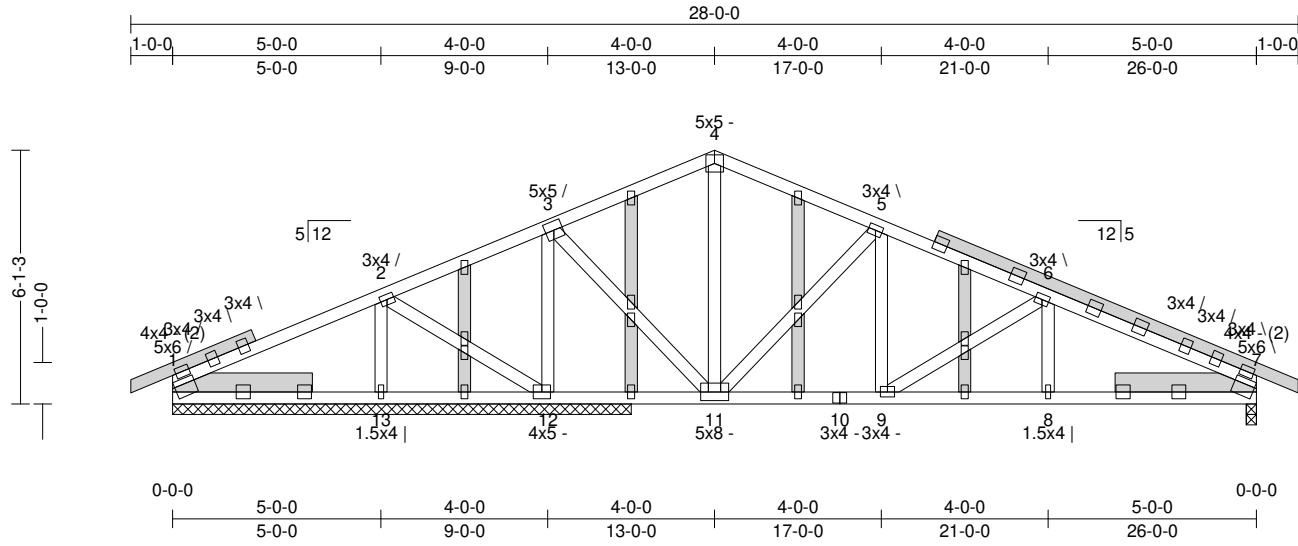
Truss: L03

Job: PP - 3410\_0 - ARisley4Bdr1S

Date: 03/17/20 07:44:45

Page: 1 of 1

SPAN 26-0-0	PITCH 5/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 151 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IRC 2015/	TC: 0.47 (2-3)	Vert TL: 0.1 in	L / 999	(8-9)	L / 240
TCDL: 15	TP1 1-2014	BC: 0.44 (7-8)	Vert LL: 0.05 in	L / 999	(8-9)	L / 360
BCLL: 0	Rep Mbr: No	Web: 0.39 (5-11)	Horz TL: 0.02 in		7	
BCDL: 10	Lumber D.O.L.: 115 %					

#### Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
7	1	3 in	1.78 in	1,135 lbs	-	-300 lbs	-418 lbs	-418 lbs	-
12	1	132 in	N/A	1,900 lbs	-	-500 lbs	-622 lbs	-622 lbs	317 lbs
13	1	132 in	N/A	269 lbs	-57 lbs	-52 lbs	-68 lbs	-68 lbs	-
1	1	132 in	N/A	123 lbs	-199 lbs	-	-	-199 lbs	233 lbs
1	1	132 in	N/A	413 lbs	-	-179 lbs	-195 lbs	-195 lbs	220 lbs
1	1	132 in	N/A	161 lbs	-	-28 lbs	-34 lbs	-34 lbs	-219 lbs

03/17/2020



#### Material

TC: SPF 1650/1.5 2 x 4  
BC: SPF 1650/1.5 2 x 4  
Web: SPF 1650/1.5 2 x 4

#### Bracing

TC: Sheathed or Purlins at 5-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 8-11-0, Purlin design by Others.

#### Loads

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/CtCs = 1, DOL = 1.15.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Partial, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Minimum storage attic loading has been applied in accordance with IRC 301.5

#### Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web
1-2 0.305 366 lbs (-194 lbs)	3-4 0.308 -421 lbs	5-6 0.316 -1,175 lbs
2-3 0.473 706 lbs (-106 lbs)	4-5 0.308 -442 lbs	6-7 0.207 -1,622 lbs
7-8 0.440 1,461 lbs (-289 lbs)	9-11 0.307 1,016 lbs (-86 lbs)	
8-9 0.395 1,461 lbs (-289 lbs)	11-12 0.131 -618 lbs	
2-12 0.092 -354 lbs	5-11 0.389 -1,015 lbs	
3-12 0.328 -1,542 lbs	5-9 0.076 469 lbs (-84 lbs)	
3-11 0.217 1,334 lbs (-302 lbs)	6-9 0.136 -523 lbs	

#### Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Upper top chord notching is permitted beyond horizontal dimension of 24.00" from the right heel.
- Gable webs placed at 24" OC, U.N.O.
- Attach structural gable blocks with 2x4 20ga plates, U.N.O.
- Stitch top chords together with 4x4 20Ga plates at 24 in oc maximum, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- A creep factor of 1.00 has been applied for this truss analysis.
- ☐ Indicates non-structural members.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 13, 1 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild@Truss Software V5.6.355  
Eagle Metal Products

1122757 0005/0016

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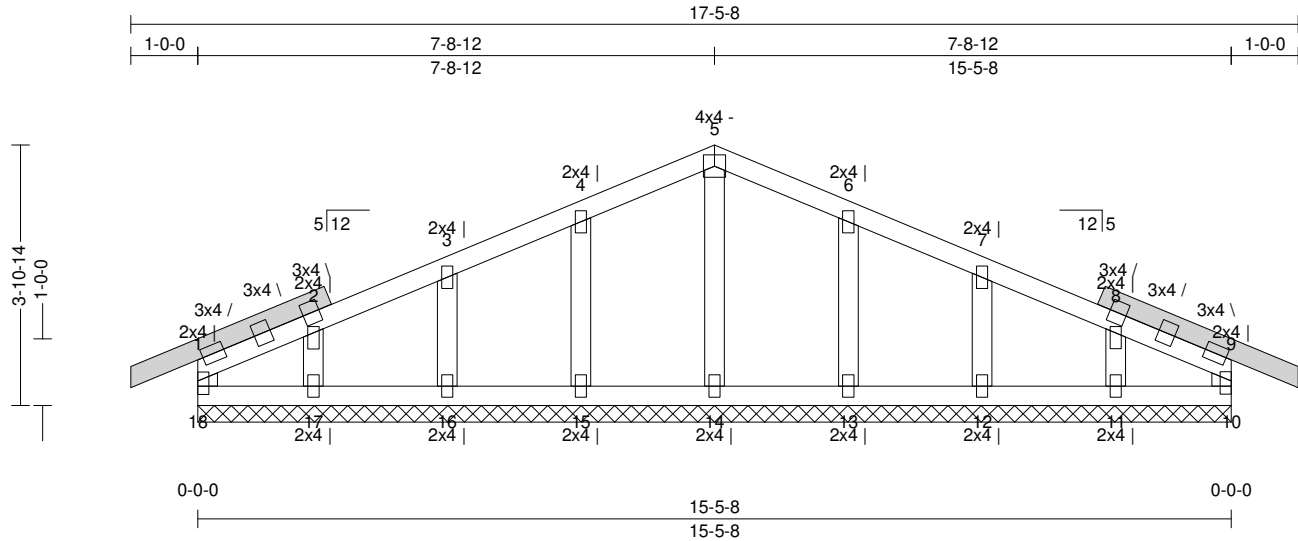
Truss: L04

Job: PP - 3410\_0 - ARisley4Bdr1S

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SPAN 15-5-8	PITCH 5/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 61 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IRC 2015/	TC: 0.21 (1-2)	Vert TL: 0 in UP	L/999	10	L/240
TCDL: 15	TP1 1-2014	BC: 0.02 (15-16)	Vert LL: 0 in	L/999	10	L/360
BCLL: 0	Rep Mbr: No	Web: 0.10 (1-18)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

03/17/2020

**Reaction**

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		305 lbs	152 plf		-94 lbs	-175 lbs	-175 lbs	-114 lbs

**Material**

TC: SPF 1650/1.5 2 x 4  
BC: SPF 1650/1.5 2 x 4  
Web: SPF 1650/1.5 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- 1) This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/CtCs = 1, DOL = 1.15.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Partial, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

**Member Forces**

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 24" OC, U.N.O.
- 4) Attach gable webs with 2x4 20ga plates, U.N.O.
- 5) Stitch top chords together with 20Ga plates at 24 in oc maximum, U.N.O.
- 6) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 7) The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- 8) A creep factor of 1.00 has been applied for this truss analysis.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.





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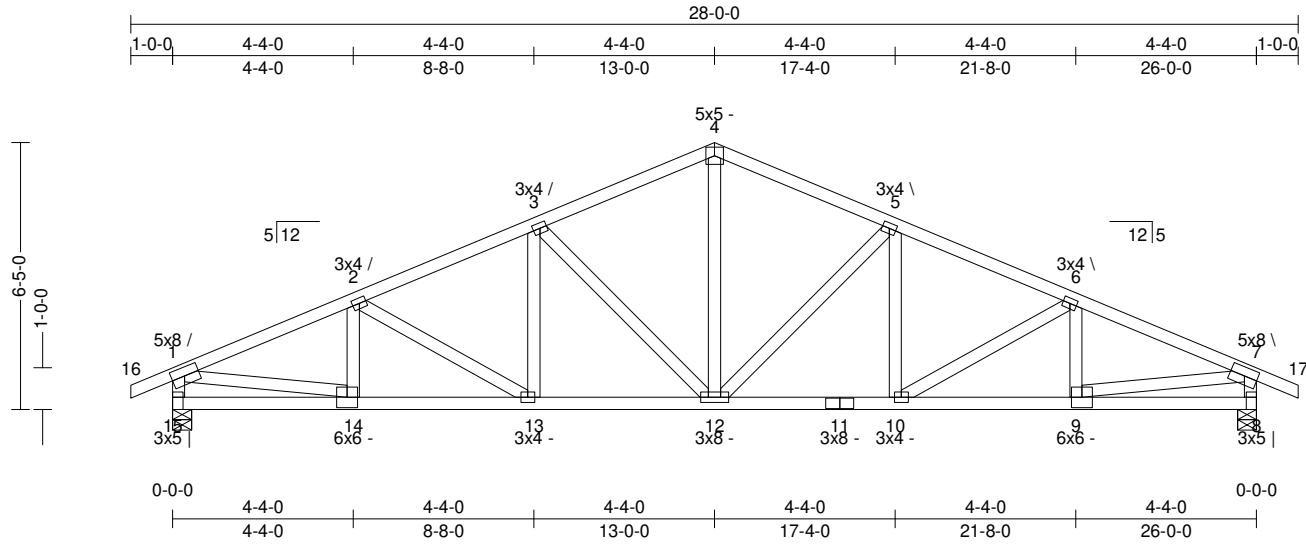
Truss: T01

Job: PP - 3410\_0 - ARisley4Bdr1S

Date: 03/17/20 07:44:53

Page: 1 of 1

SPAN 26-0-0	PITCH 5/12	QTY 12	OHL 1-0-0	OHR 1-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 119 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IRC 2015/	TC: 0.32 (5-6)	Vert TL: 0.2 in	L/999	(12-13)	L/240
TCDL: 15	TP1 1-2014	BC: 0.45 (10-12)	Vert LL: 0.09 in	L/999	12	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.40 (7-9)	Horz TL: 0.06 in		8	
BCDL: 10	Lumber D.O.L.: 115 %					

#### Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	5.5 in	2.84 in	1,813 lbs		-298 lbs	-462 lbs	-462 lbs	-33 lbs
8	1	5.5 in	2.84 in	1,813 lbs		-298 lbs	-462 lbs	-462 lbs	

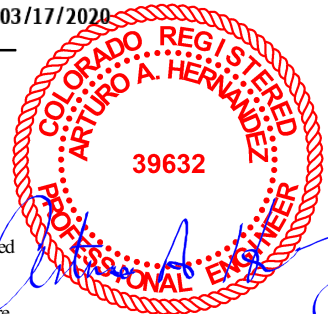
03/17/2020

#### Material

TC: SPF 1650/1.5 2 x 4  
BC: SPF 1650/1.5 2 x 4  
Web: SPF 1650/1.5 2 x 4

#### Bracing

TC: Sheathed or Purlins at 4-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



#### Loads

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/Cts = 1, DOL = 1.15.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Minimum storage attic loading has been applied in accordance with IRC 301.5

#### Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.256	-2,712 lbs	3-4	0.304	-2,004 lbs	5-6	0.318	-2,562 lbs
	2-3	0.318	-2,562 lbs	4-5	0.304	-2,004 lbs	6-7	0.256	-2,712 lbs
BC	9-10	0.450	2,443 lbs	12-13	0.453	2,291 lbs			
	10-12	0.453	2,291 lbs	13-14	0.450	2,443 lbs			
Web	1-15	0.170	-1,734 lbs	5-12	0.358	-828 lbs			
	1-14	0.403	2,483 lbs	7-9	0.403	2,483 lbs			
	3-12	0.358	-828 lbs	7-8	0.170	-1,734 lbs			
	4-12	0.176	1,082 lbs						

#### Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.00 has been applied for this truss analysis.
- Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

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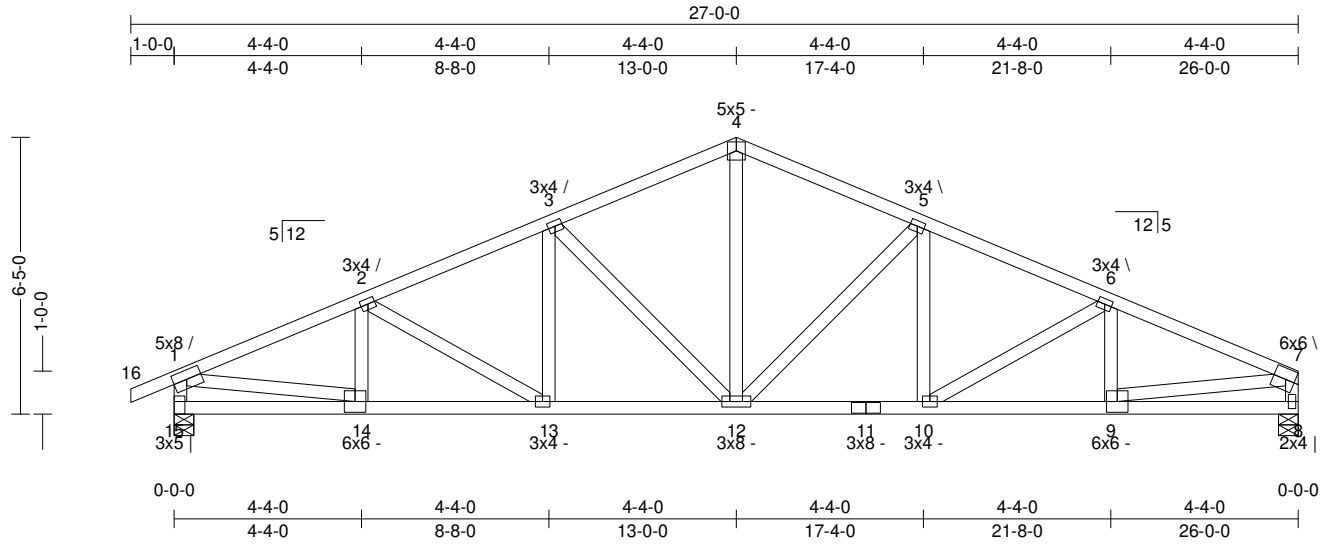
Truss: T02

Job: PP - 3410\_0 - ARisley4Bdr1S

Date: 03/17/20 07:44:57

Page: 1 of 1

SPAN 26-0-0	PITCH 5/12	QTY 12	OHL 1-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 117 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IRC 2015/	TC: 0.32 (2-3)	Vert TL: 0.2 in	L/999	(11-12)	L/240
TCDL: 15	TP1 1-2014	BC: 0.45 (10-12)	Vert LL: 0.09 in	L/999	(11-12)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.41 (7-9)	Horz TL: 0.06 in		8	
BCDL: 10	Lumber D.O.L.: 115 %					

#### Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	5.5 in	2.85 in	1,814 lbs		-298 lbs	-463 lbs	-463 lbs	53 lbs
8	1	5.5 in	2.70 in	1,721 lbs		-265 lbs	-378 lbs	-378 lbs	

#### Material

TC: SPF 1650/1.5 2 x 4  
BC: SPF 1650/1.5 2 x 4  
Web: SPF 1650/1.5 2 x 4

#### Bracing

TC: Sheathed or Purlins at 4-1-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

#### Loads

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/CtCs = 1, DOL = 1.15.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Minimum storage attic loading has been applied in accordance with IRC 301.5

#### Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

Member	ID	Force	Force	Force	Force	Force	Force		
TC	1-2	0.256	-2,715 lbs	3-4	0.304	-2,009 lbs	5-6	0.314	-2,572 lbs
	2-3	0.318	-2,564 lbs	4-5	0.299	-2,009 lbs	6-7	0.296	-2,733 lbs
BC	9-10	0.454	2,468 lbs (-474 lbs)	12-13	0.453	2,293 lbs (-377 lbs)			
	10-12	0.454	2,299 lbs (-379 lbs)	13-14	0.450	2,445 lbs (-459 lbs)			
Web	1-15	0.170	-1,735 lbs	5-12	0.360	-832 lbs			
	1-14	0.403	2,485 lbs (-445 lbs)	7-9	0.407	2,508 lbs (-503 lbs)			
	3-12	0.358	-827 lbs	7-8	0.161	-1,641 lbs			
	4-12	0.176	1,085 lbs (-259 lbs)						

#### Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.00 has been applied for this truss analysis.
- Listed wind uplift reactions based on MWFRS & C&C loading.





### Habitat For Humanity

6564 State Hwy. 96  
Olney Springs ,CO 81062  
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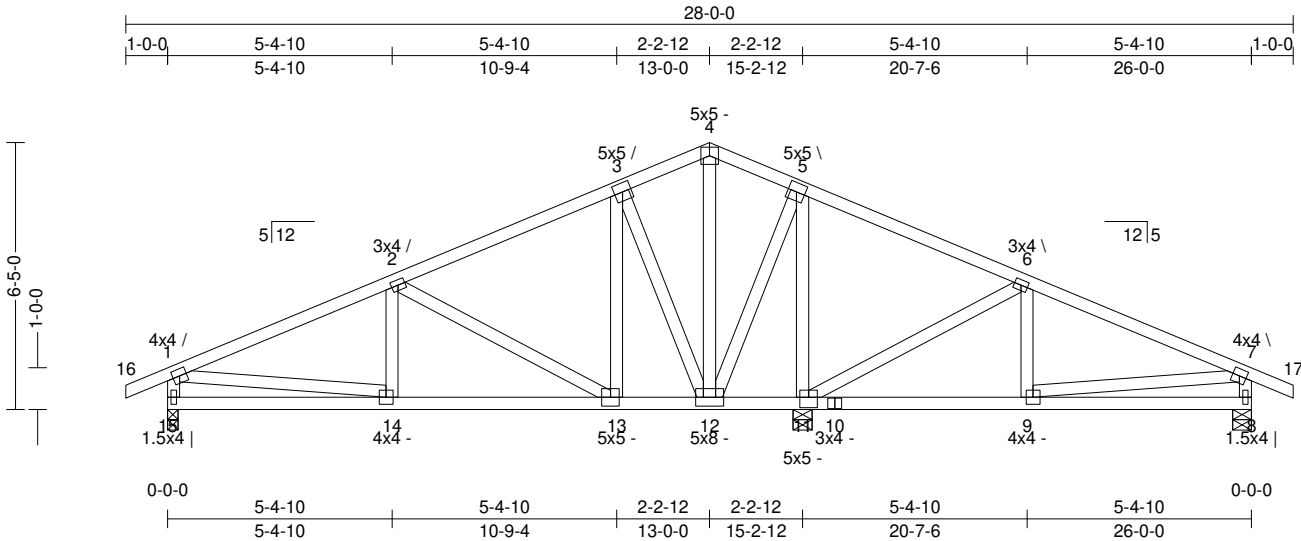
Truss: T03

Job: PP - 3410\_0-Arisley4Bdr1S

Date: 03/17/20 07:45:01

Page: 1 of 1

SPAN 26-0-0	PITCH 5/12	QTY 2	OHL 1-0-0	OHR 1-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 124 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IRC 2015/	TC: 0.66 (5-6)	Vert TL: 0.06 in	L/999	(8-9)	L/240
TCDL: 15	TP1 1-2014	BC: 0.40 (13-14)	Vert LL: 0.02 in	L/999	(8-9)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.48 (5-11)	Horz TL: 0.01 in		8	
BCDL: 10	Lumber D.O.L.: 115 %					

#### Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	3 in	1.59 in	1,015 lbs	-	-256 lbs	-370 lbs	-370 lbs	-33 lbs
11	1	5.5 in	3.28 in	2,094 lbs	-	-569 lbs	-692 lbs	-692 lbs	-
8	1	5.5 in	1.50 in	624 lbs	-	-157 lbs	-250 lbs	-250 lbs	-

03/17/2020

#### Material

TC: SPF 1650/1.5 2 x 4  
BC: SPF 1650/1.5 2 x 4  
Web: SPF 1650/1.5 2 x 4

#### Bracing

TC: Sheathed or Purlins at 5-11-0, Purlin design by Others.  
BC: Sheathed or Purlins at 9-11-0, Purlin design by Others.

#### Loads

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/Cs = 1, DOL = 1.15.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Partial, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Minimum storage attic loading has been applied in accordance with IRC 301.5

#### Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

Member	CSI	Max Axial Force	Max Axial Force	Max Axial Force	Max Axial Force	Max Axial Force	Max Axial Force	Max Axial Force	Max Axial Force	
TC 1-2	0.474	-1,291 lbs	5-6	0.663	594 lbs				(-68 lbs)	
TC 2-3	0.468	-570 lbs	6-7	0.477	-538 lbs					
BC 9-11	0.290	429 lbs	(5 lbs)	12-13	0.195	410 lbs				
BC 11-12	0.157	-469 lbs		13-14	0.403	1,123 lbs			(-160 lbs)	
Web 1-15	0.091	-925 lbs		3-12	0.404	-1,076 lbs	7-9	0.070	433 lbs	(-5 lbs)
Web 1-14	0.184	1,135 lbs	(-183 lbs)	5-12	0.182	1,120 lbs	7-8	0.052	-534 lbs	
Web 2-13	0.355	-813 lbs		5-11	0.476	-1,478 lbs				
Web 3-13	0.093	570 lbs	(-116 lbs)	6-11	0.405	-926 lbs				

#### Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.00 has been applied for this truss analysis.
- Listed wind uplift reactions based on MWFRS & C&C loading.



**Habitat For Humanity**

6564 State Hwy. 96  
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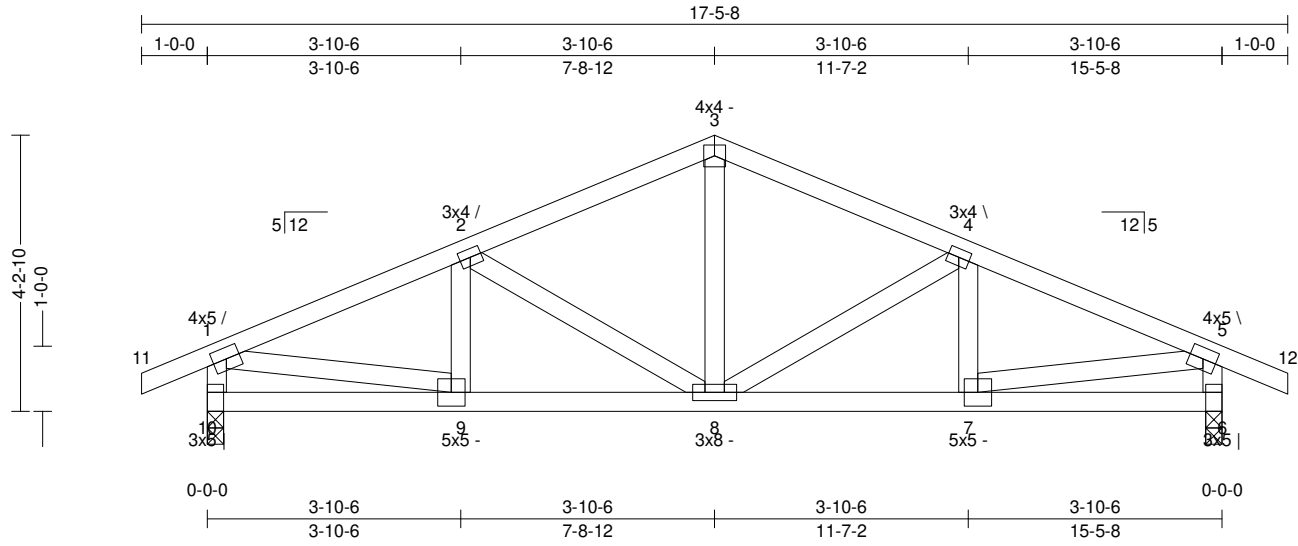
Truss: T04

Job: PP - 3410\_0-Arisley4Bdr1S

Date: 03/17/20 07:45:05

Page: 1 of 1

SPAN 15-5-8	PITCH 5/12	QTY 1	OHL 1-0-0	OHR 1-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 66 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IRC 2015/	TC: 0.27 (4-5)	Vert TL: 0.06 in	L/999	(7-8)	L/240
TCDL: 15	TP1 1-2014	BC: 0.32 (7-8)	Vert LL: 0.03 in UP	L/999	(7-8)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.23 (5-7)	Horz TL: 0.01 in		6	
BCDL: 10	Lumber D.O.L.: 115 %					

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
10	1	3 in	1.90 in	1,209 lbs	-	-311 lbs	-624 lbs	-624 lbs	-33 lbs
6	1	3 in	1.90 in	1,209 lbs	-	-311 lbs	-624 lbs	-624 lbs	-

**Material**

TC: SPF 1650/1.5 2 x 4  
BC: SPF 1650/1.5 2 x 4  
Web: SPF 1650/1.5 2 x 4

**Bracing**

TC: Sheathed or Purlins at 5-5-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL = 1.15.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Partial, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered, DOL = 1.60
- Minimum storage attic loading has been applied in accordance with IRC 301.5

**Member Forces**

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

Member	Force 1	Force 2	Force 3	Force 4	Force 5	Force 6	Force 7	Force 8	Force 9
TC	1-2	0.274	-1,553 lbs	3-4	0.271	-1,151 lbs			
	2-3	0.271	-1,151 lbs	4-5	0.274	-1,553 lbs			
BC	7-8	0.324	1,367 lbs	(-488 lbs)	8-9	0.324	1,367 lbs	(-488 lbs)	
Web	1-10	0.111	-1,138 lbs	3-8	0.075	-459 lbs	(-209 lbs)	5-6	0.111
	1-9	0.227	1,395 lbs	(-530 lbs)	4-8	0.111	-462 lbs		
	2-8	0.111	-462 lbs	5-7	0.227	1,395 lbs	(-530 lbs)		

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.00 has been applied for this truss analysis.
- Listed wind uplift reactions based on MWFRS & C&C loading.



### Habitat For Humanity

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Olney Springs ,CO 81062  
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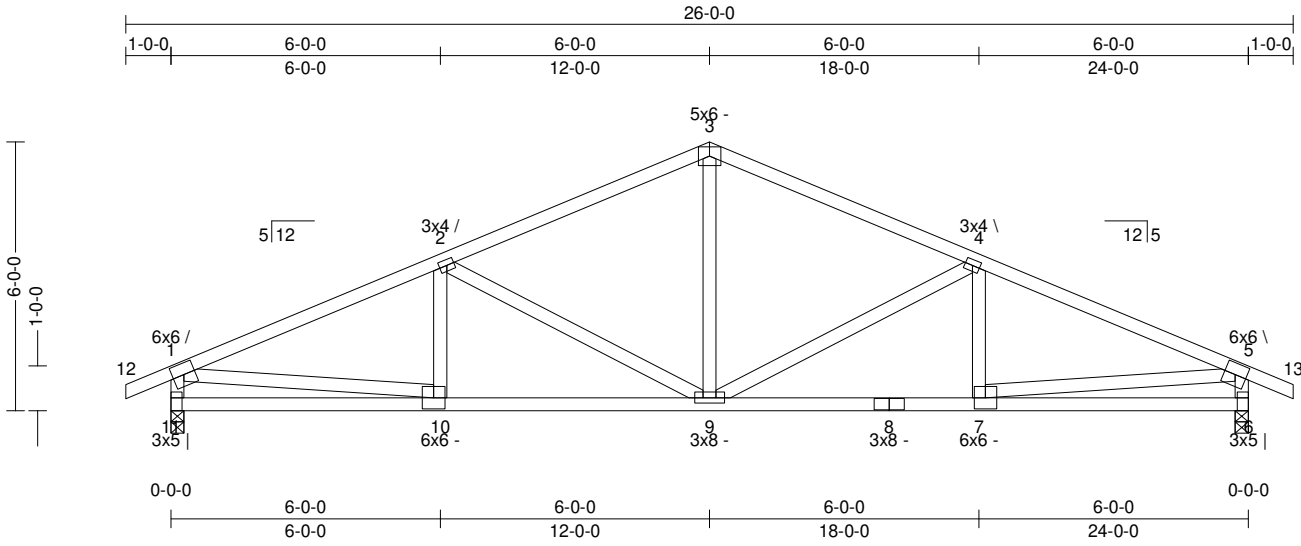
Truss: T05

Job: PP - 3410\_0 - Arisley4Bdr1S

Date: 03/17/20 07:45:10

Page: 1 of 1

SPAN 24-0-0	PITCH 5/12	QTY 6	OHL 1-0-0	OHR 1-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 101 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30 TCDL: 15 BCLL: 0 BCDL: 10	Bldg Code: IRC 2015/ TP1 1-2014 Rep Mbr: Yes Lumber D.O.L.: 115 %	TC: 0.57 (4-5) BC: 0.56 (7-9) Web: 0.45 (4-9)	Vert TL: 0.2 in Vert LL: 0.08 in Horz TL: 0.04 in	L/999 L/999	(8-9) (8-9) 6	L/240 L/360

#### Reaction

03/17/2020

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
11	1	3.5 in	2.66 in	1,697 lbs	-	-277 lbs	-433 lbs	-433 lbs	-33 lbs
6	1	3.5 in	2.66 in	1,697 lbs	-	-277 lbs	-433 lbs	-433 lbs	-

#### Material

TC: SPF 1650/1.5 2 x 4  
BC: SPF 1650/1.5 2 x 4  
Web: SPF 1650/1.5 2 x 4

#### Bracing

TC: Sheathed or Purlins at 4-1-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



#### Loads

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/CtCs = 1, DOL = 1.15.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Minimum storage attic loading has been applied in accordance with IRC 301.5

#### Member Forces

Table indicates: Member ID, max CSI max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

Member	Force 1	Force 2	Force 3	Force 4	Force 5	Force 6	Force 7	Force 8	Force 9
TC	1-2	0.565	-2,619 lbs	3-4	0.558	-1,903 lbs			
	2-3	0.558	-1,903 lbs	4-5	0.565	-2,619 lbs			
BC	7-9	0.556	2,342 lbs	(-366 lbs)	9-10	0.556	2,342 lbs	(-366 lbs)	
Web	1-11	0.156	-1,594 lbs	3-9	0.138	852 lbs	(-150 lbs)	5-6	0.156
	1-10	0.384	2,362 lbs	(-391 lbs)	4-9	0.452	-842 lbs		
	2-9	0.452	-842 lbs	5-7	0.384	2,362 lbs	(-391 lbs)		

#### Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.00 has been applied for this truss analysis.
- Listed wind uplift reactions based on MWFRS & C&C loading.

**Habitat for Humanity of Colorado**

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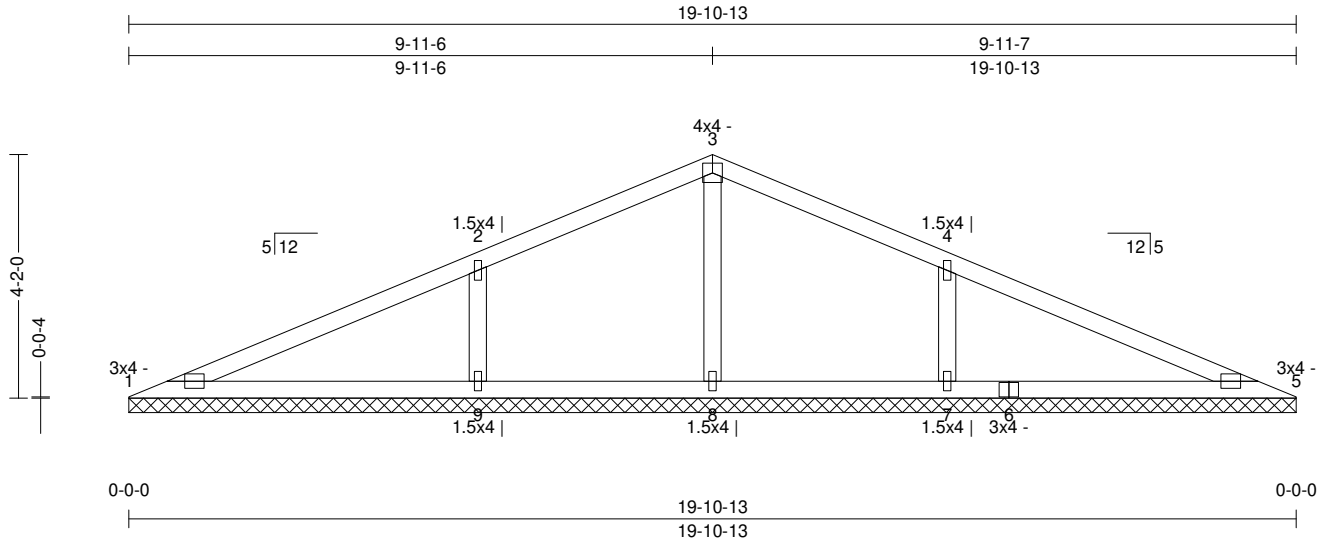
Truss: V01

Job: PP - 3410\_0 - ARisley4Bdr1S

Date: 03/17/20 07:45:14

Page: 1 of 1

SPAN 19-10-13	PITCH 5/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 52 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IRC 2015/	TC: 0.36 (1-2)	Vert TL: 0.01 in	L/999	(9-1)	L/240
TCDL: 15	TP1 1-2014	BC: 0.17 (9-1)	Vert LL: 0 in	L/999	(9-1)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.07 (2-9)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

**Reaction**

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		687 lbs	164 plf	-165 lbs	-160 lbs	-214 lbs	-214 lbs	345 lbs

**Material**

TC: SPF 1650/1.5 2 x 4  
 BC: SPF 1650/1.5 2 x 4  
 Web: SPF 1650/1.5 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL = 1.15.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

**Member Forces**

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web
29	0.065	-591 lbs
4-7	0.065	-590 lbs

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 48" OC, U.N.O.
- Attach gable webs with 1.5x4 20ga plates, U.N.O.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 1.00 has been applied for this truss analysis.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 1, 5, 1, 5 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.



**Habitat for Humanity of Colorado**

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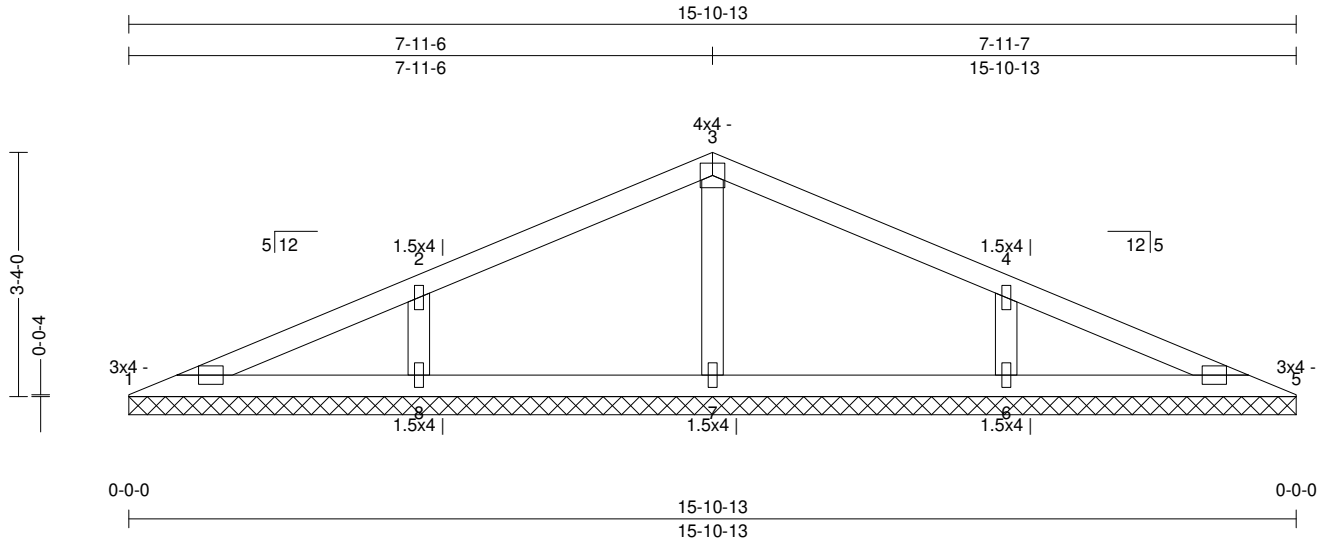
Truss: V02

Job: PP - 3410\_0-Arisley4Bdr1S

Date: 03/17/20 07:45:18

Page: 1 of 1

SPAN 15-10-13	PITCH 5/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 41 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IRC 2015/	TC: 0.25 (4-5)	Vert TL: 0 in	L/999	(6-7)	L/240
TCDL: 15	TP1 1-2014	BC: 0.08 (6-7)	Vert LL: 0 in	L/999	5	L/360
BCLL: 0	Rep Mbr: No	Web: 0.05 (4-6)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

03/17/2020

**Reaction**

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	558 lbs	141 plf	-23 lbs	-105 lbs	-244 lbs	-244 lbs	140 lbs

**Material**

TC: SPF 1650/1.5 2 x 4  
 BC: SPF 1650/1.5 2 x 4  
 Web: SPF 1650/1.5 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL = 1.15.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

**Member Forces**

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web	2-8	0.049	-484 lbs	3-7	0.045	-325 lbs	4-6	0.049	-484 lbs

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 48" OC, U.N.O.
- Attach gable webs with 1.5x4 20ga plates, U.N.O.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 1.00 has been applied for this truss analysis.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 1, 5, 1, 5 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.



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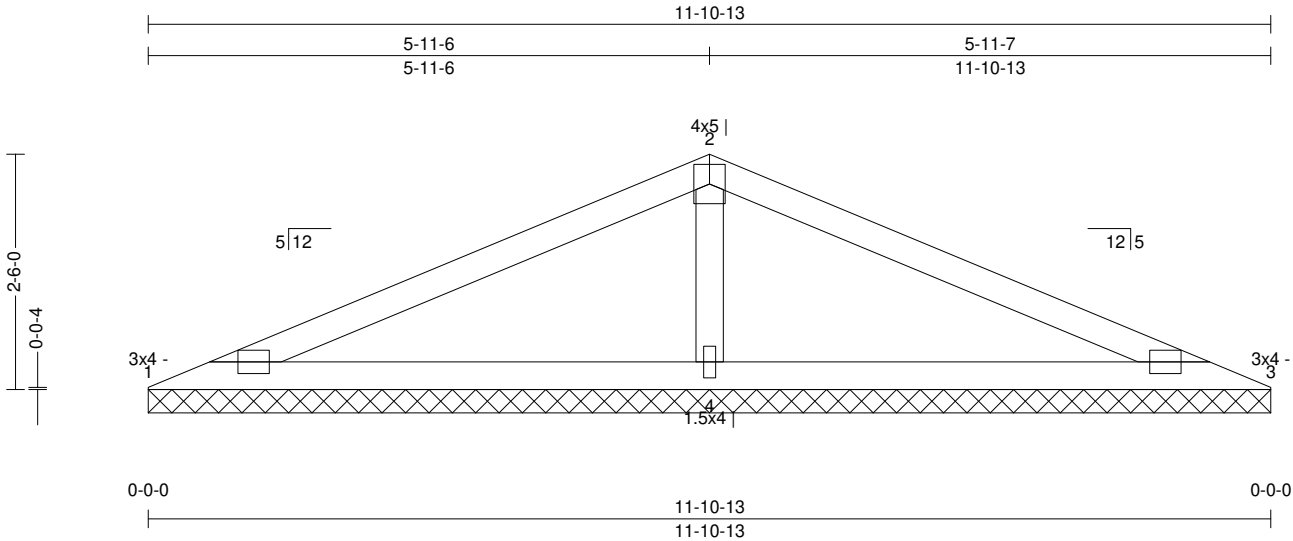
Truss: V03

Job: PP - 3410\_0-Arisley4Bdr1S

Date: 03/17/20 07:45:21

Page: 1 of 1

SPAN 11-10-13	PITCH 5/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 28 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IRC 2015/	TC: 0.48 (2-3)	Vert TL: 0.01 in	L/999	(3-4)	L/240
TCDL: 15	TPI 1-2014	BC: 0.21 (3-4)	Vert LL: 0.01 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.04 (2-4)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

**Reaction**

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	863 lbs	207 plf	-204 lbs	-152 lbs	-391 lbs	-391 lbs	459 lbs

**Material**

TC: SPF 1650/1.5 2 x 4  
 BC: SPF 1650/1.5 2 x 4  
 Web: SPF 1650/1.5 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/CtCs = 1, DOL = 1.15.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

**Member Forces**

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

Member	Force 1	Force 2	Force 3	Force 4	Force 5	Force 6		
TC	1-2	0.485	360 lbs	(-278 lbs)	2-3	0.485	360 lbs	(-278 lbs)
BC								
Web	2-4	0.037	-335 lbs					

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 48" OC, U.N.O.
- Attach gable webs with 3x4 20ga plates, U.N.O.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 1.00 has been applied for this truss analysis.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 1, 3, 1, 3 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.

03/17/2020





**Habitat for Humanity of Colorado**

P.O. Box 100  
 Onley Springs, CO 81062  
 (719) 267-5323

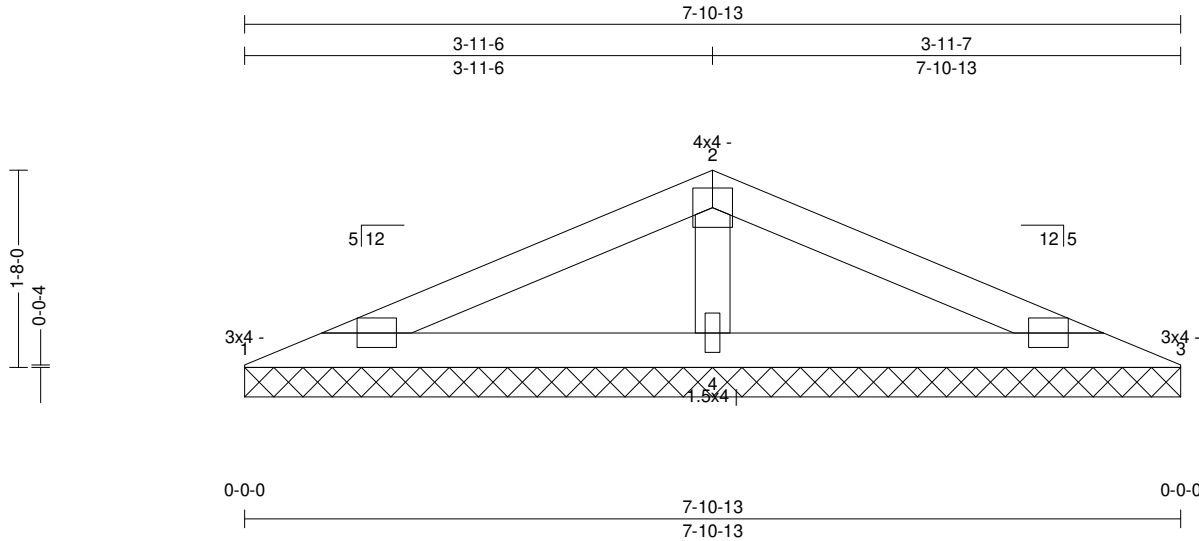
Truss: V04

Job: PP - 3410\_0 - ARisley4Bdr1S

Date: 03/17/20 07:45:25

Page: 1 of 1

SPAN 7-10-13	PITCH 5/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 18 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IRC 2015/	TC: 0.16 (2-3)	Vert TL: 0 in	L/999	(3-4)	L/240
TCDL: 15	TPI 1-2014	BC: 0.06 (3-4)	Vert LL: 0 in	L/999	3	L/360
BCLL: 0	Rep Mbr: No	Web: 0.02 (2-4)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

03/17/2020

**Reaction**

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		414 lbs	153 plf	-49 lbs	-76 lbs	-219 lbs	-219 lbs	188 lbs

**Material**

TC: SPF 1650/1.5 2 x 4  
 BC: SPF 1650/1.5 2 x 4  
 Web: SPF 1650/1.5 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL = 1.15.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

**Member Forces**

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 48" OC, U.N.O.
- Attach gable webs with 3x4 20ga plates, U.N.O.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 1.00 has been applied for this truss analysis.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 1, 3, 1, 3 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.



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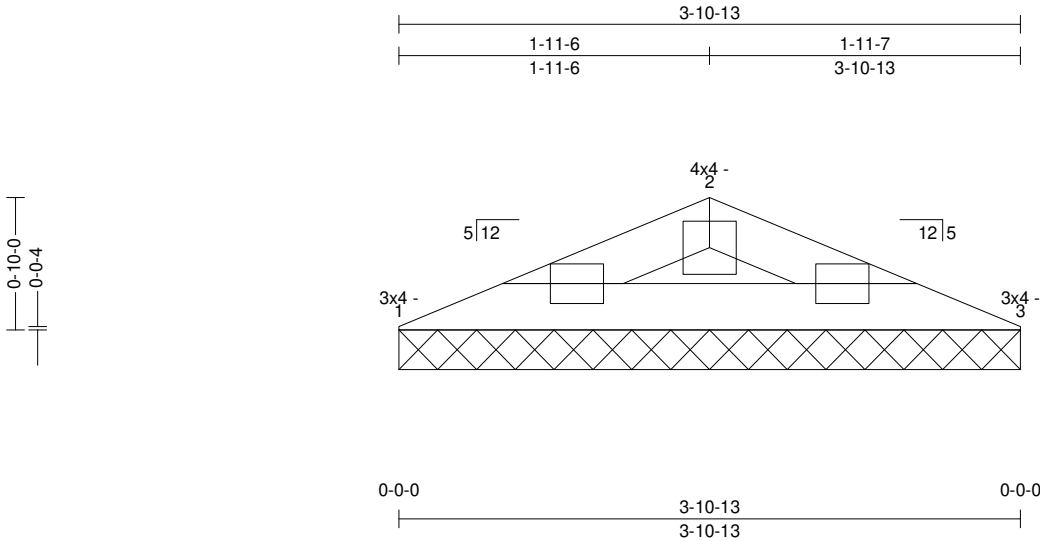
Truss: V05

Job: PP - 3410\_0 - ARisley4Bdr1S

Date: 03/17/20 07:45:27

Page: 1 of 1

SPAN 3-10-13	PITCH 5/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 8 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IRC 2015/	TC: 0.02 (2-3)	Vert TL: 0 in	L/999	3	L/240
TCDL: 15	TP1 1-2014	BC: 0.01 (3-1)	Vert LL: 0 in	L/999	3	L/360
BCLL: 0	Rep Mbr: No	Web: 0.00 (1)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

03/17/2020

**Reaction**

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		155 lbs	110 plf		-34 lbs	-113 lbs	-113 lbs	-106 lbs

**Material**

TC: SPF 1650/1.5 2 x 4  
 BC: SPF 1650/1.5 2 x 4  
 Web:

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- 1) This truss has been designed for the effects of balanced (30 psf) roof snow loads, in accordance with ASCE7 - 10 except as noted, with the following user defined input: 30 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

**Member Forces**

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 48" OC, U.N.O.
- 4) Attach gable webs with 3x4 20ga plates, U.N.O.
- 5) The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- 6) A creep factor of 1.00 has been applied for this truss analysis.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

