EAGLE METAL

12300 Ford Rd, Suite 110
Dallas, Texas 75234

eaglemetal.com

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Habitat for Humanity of Colorado**.

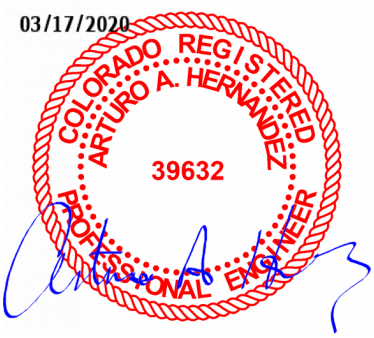
These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: **PP - 3410_0 - B Risley4Bdr1S** - 1122758
G01, G02, J01, J02, L01, L02, L03, T01, T02, T03, T04, V01, V02, V03, V04, V05

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.



Arturo A. Hernandez (CO, PE-39632)
My license renewal date for the state of CO is 10/31/2021.

IMPORTANT NOTE: The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

⚠ **WARNING:** Failure to follow may result in property damage or personal injury.

DESIGN NOTES

1. The Truss Design Drawing(s) provided with these General Notes have been prepared under and are subject to ANSI/TPI1. Capitalized terms have the meanings provided in ANSI/TPI1.
2. Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.

DESIGN LIMITATIONS

3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANS1/TPI1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
6. Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

HANDLING, INSTALLING, & BRACING

7. Refer to BCSI for handling, installing, restraining and bracing trusses. Copies can be obtained from the Truss Plate Institute (TPI), 218 N Lee Street, Suite 312, Alexandria, VA 22314, www.tpinst.org or SBCA, 6300 Enterprise Lane, Madison, WI 53719, www.sbcindustry.com.
8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
9. Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
10. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.
11. Bottom chord required bracing shall be at 1Oft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
12. Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
13. Never exceed the design loading shown and never stack building or other materials on inadequately braced truss; refer to BCSI.
14. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
15. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

MATERIALS & FABRICATION

16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
17. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
18. Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI/TPI1.
20. For a specified plate gauge and grade, the specified size is a minimum.
21. Connections not shown are the responsibility of others.
22. Adequate support shall be provided to resist gravity, lateral, uplift loads.
23. For 4X2 truss orientation, locate plates 0 - 1/16" from outside the edge of the truss.
24. Fabrication of truss shall be in accordance with ANSI/TPI1.

OTHER NOTES

25. Camber is a non-structural consideration and is the responsibility of truss fabricator.
26. Do not cut or alter any truss member or plate without prior approval from a professional engineer.
27. Lumber design values are in accordance with ANSI/TPI; lumber design values are by others.
28. Install specified hangers per manufacturer recommendations.

SYMBOLS

PLATE SIZE

3X4 - The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

-, /, I, Indicates required direction of slots; Reference "Joint Details" for more information.

20 Ga Gr40 connectors required
3X10-20HS - 20 Ga Gr60 connectors required
8X10-18HS - 18 Ga Gr60 connectors required

LATERAL BRACING

When this symbol shown, continuous lateral bracing is required on the web of the truss.

BEARING

Indicates location where bearings (supports) occur.

PLATE LOCATION & ORIENTATION

The plate shall be centered on joint and/or placed in accordance with the design drawing/QC full scale details.

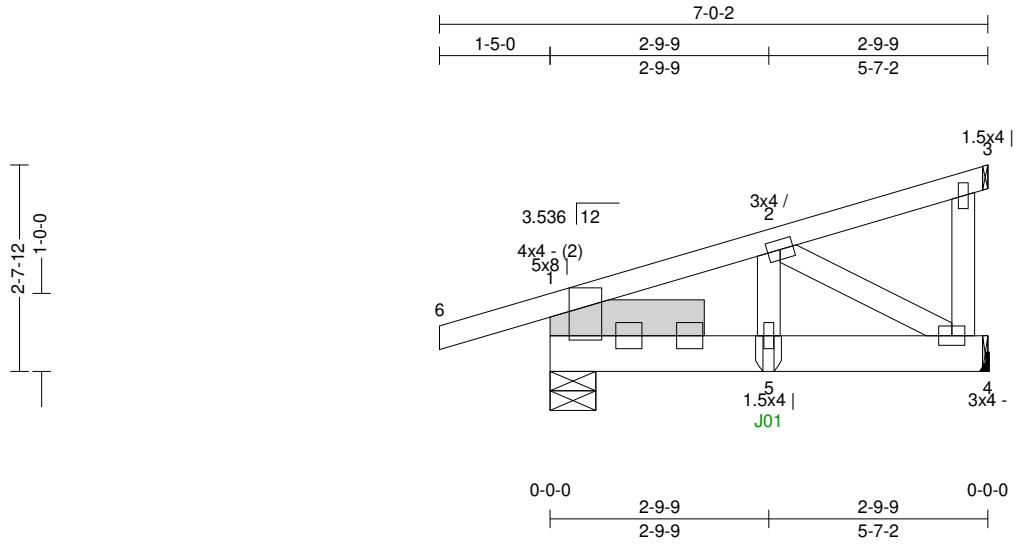
REFERENCES

- **ANSI/TPI1:** National Design Standard for Metal Plate Connected Wood Trusses
- **BCSI:** Building Component & Safety Information - Guide to Good Practice for Handling, Installing, Restraining, & Bracing of Metal Plate Connected Wood Trusses.
- **NDS:** National Design Specification for Wood Construction
- **ESR:** 1082 published by the International Code Council. www.icc-es.org

Habitat For Humanity
6564 State Hwy. 96
Olney Springs ,CO 81062
(719) 267-5323

Truss: G01
Job: PP - 3410_0 - B Risley4Bdr1S
Date: 03/17/20 07:45:31
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
5-7-2	3.536/12	1	1-5-0	0-0-0	0-0-0	0-0-0	1	24 in	29 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code :	IRC 2015/	TC :	0.22 (6-1)	Vert TL:	0.01 in	L / 999	(4-5)	L / 240
TCLL : 30		TPI 1-2014	BC :	0.08 (4-5)	Vert LL:	0 in	L / 999	(4-5)	L / 360
TCDL : 15	Rep Mbr :	No	Web :	0.08 (3-4)	Horz TL:	0 in		4	
BCLL : 0	Lumber D.O.L. :	115 %							
BCDL : 10									

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	7.028 in	1.50 in	469 lbs	·	-194 lbs	-397 lbs	-397 lbs	135 lbs
4	1	1.5 in	---	341 lbs	·	-209 lbs	-241 lbs	-241 lbs	·

03/17/2020
COLORADO REGISTERED ARTURO A. HERNANDEZ 39632 PROFESSIONAL ENGINEER

Material

TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 6
Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (30 psf) roof snow loads. in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL= 1.15.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Minimum storage attic loading has been applied in accordance with IRC 301.5

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-1-5-0	-0-4-6	Down	Proj	0 plf	32.48 plf	
Top	-0-4-6	2-10-4	Down	Proj	32.48 plf	0 plf	
Top	-1-5-0	-0-4-6	Down	Proj	0 plf	32.48 plf	
Top	-0-4-6	2-10-4	Down	Proj	32.48 plf	0 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-1-5-0	-0-4-6	Down	Proj	0 plf	16.24 plf	
Top	-0-4-6	2-10-4	Down	Proj	16.24 plf	0 plf	
Top	-1-5-0	-0-4-6	Down	Proj	0 plf	16.24 plf	
Top	-0-4-6	2-10-4	Down	Proj	16.24 plf	0 plf	

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.213	-393 lbs						
BC	4-5	0.079	353 lbs	(-210 lbs)	5-1	0.078	353 lbs	(-210 lbs)	
Web	2-4	0.051	-408 lbs						

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Eagle Metal Products

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
5-7-2	3.536/12	1	1-5-0	0-0-0	0-0-0	0-0-0	1	24 in	29 lbs

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
J01	BC	2-9-8
J01	BC	2-9-8

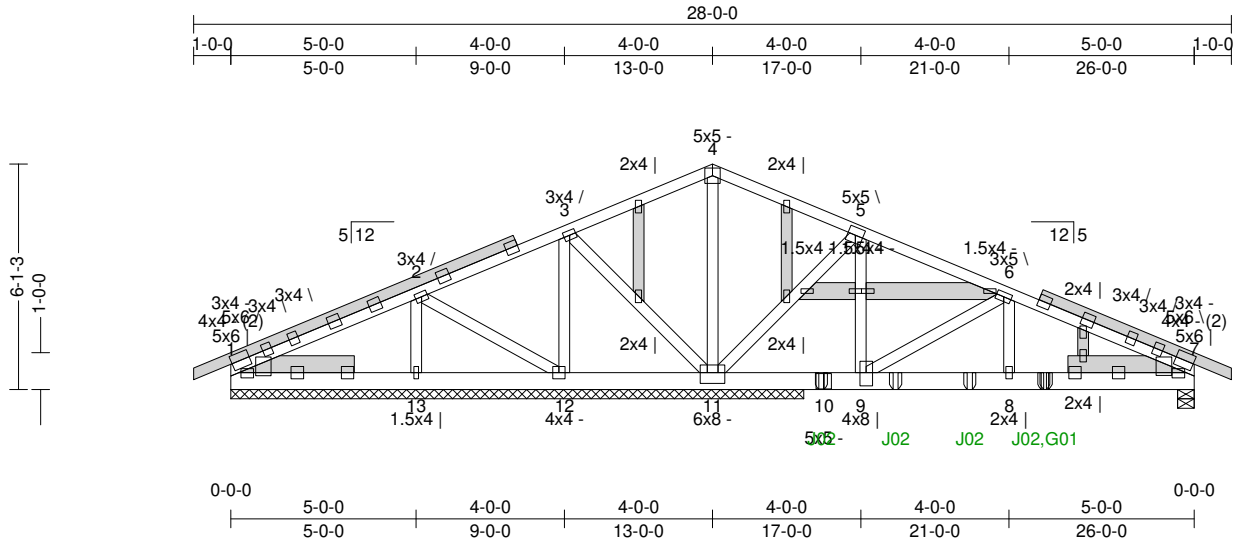
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hanger is for graphical intrepretation only. Install hanger per manufacturer's recommendation.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) A creep factor of 1.00 has been applied for this truss analysis.
6) ▭ Indicates non-structural members.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

Habitat For Humanity
6564 State Hwy. 96
Olney Springs ,CO 81062
(719) 267-5323

Truss: G02
Job: PP - 3410_0 - B Risley4Bdr1S
Date: 03/17/20 07:45:34
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-0-0	5/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	168 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code :	IRC 2015/	TC :	0.44 (4-5)	Vert TL:	0.06 in	L / 999	(8-9)	L / 240
TCLL : 30		TPI 1-2014	BC :	0.32 (7-8)	Vert LL:	0.03 in	L / 999	(7-8)	L / 360
TCDL : 15	Rep Mbr :	No	Web :	0.56 (5-11)	Horz TL:	0.01 in		7	
BCLL : 0	Lumber D.O.L. :	115 %							
BCDL : 10									

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
7	1	5.5 in	1.85 in	1,178 lbs	·	-399 lbs	-194 lbs	-399 lbs	·
11	1	185.5 in	N/A	2,454 lbs	·	-816 lbs	-295 lbs	-816 lbs	412 lbs
12	1	185.5 in	N/A	397 lbs	-10 lbs	-49 lbs	-150 lbs	-150 lbs	-225 lbs
13	1	185.5 in	N/A	311 lbs	-3 lbs	-31 lbs	-120 lbs	-120 lbs	·
1	1	185.5 in	N/A	131 lbs	-107 lbs	-4 lbs	-83 lbs	-107 lbs	163 lbs
1	1	185.5 in	N/A	281 lbs	·	-106 lbs	-121 lbs	-121 lbs	139 lbs
1	1	185.5 in	N/A	127 lbs	·	-36 lbs	-21 lbs	-36 lbs	-147 lbs

03/17/2020

COLORADO REGISTERED ARTURO A. HERNANDEZ 39632 PROFESSIONAL ENGINEER

Material

TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 6
Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 5-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL= 1.15.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
4) Minimum storage attic loading has been applied in accordance with IRC 301.5

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	26-0-0	Down	Proj	30 plf	30 plf	
Top	23-0-0	26-0-0	Down	Proj	30 plf	30 plf	
Top	0-0-0	14-11-4	Down	Proj	8.44 plf	8.44 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	26-0-0	Down	Proj	15 plf	15 plf	
Top	23-0-0	26-0-0	Down	Proj	15 plf	15 plf	
Top	0-0-0	14-11-4	Down	Proj	4.22 plf	4.22 plf	
Bot	0-0-0	26-0-0	Down	Proj	10 plf	10 plf	
Bot	23-0-0	26-0-0	Down	Proj	10 plf	10 plf	
Bot	0-0-0	14-11-4	Down	Proj	2.81 plf	2.81 plf	

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.409	508 lbs	(-142 lbs)	4-5	0.435	793 lbs	(-178 lbs)	6-7	0.215	-1,682 lbs
	3-4	0.414	794 lbs	(-179 lbs)	5-6	0.274	-487 lbs				

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Eagle Metal Products

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-0-0	5/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	168 lbs

BC	7-8	0.320	1,538 lbs	(-447 lbs)	9-11	0.164	399 lbs	(-24 lbs)
	8-9	0.296	1,538 lbs	(-447 lbs)				
Web	3-11	0.139	-377 lbs		5-9	0.199	1,228 lbs	(-372 lbs)
	4-11	0.331	-877 lbs		6-9	0.334	-1,325 lbs	
	5-11	0.563	-1,530 lbs		6-8	0.120	742 lbs	(-242 lbs)

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
J02	BC	15-11-4
J02	BC	17-11-4
J02	BC	19-11-4
J02	BC	21-11-4
G01	BC	22-0-0

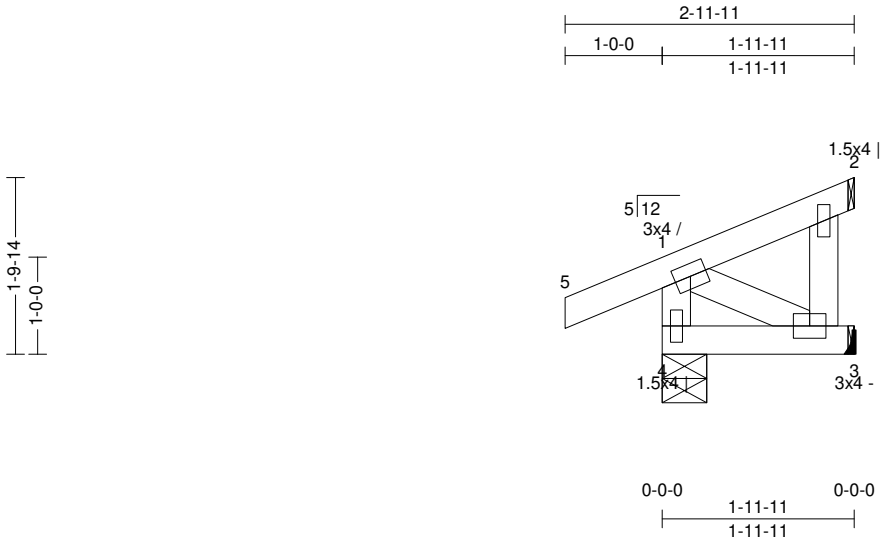
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Upper top chord notching is permitted beyond horizontal dimension of 24.00" from the left heel.
3) Upper top chord notching is permitted beyond horizontal dimension of 24.00" from the right heel.
4) Gable webs placed at 24 "OC, U.N.O.
5) Attach structural gable blocks with 4x4 20ga plates, U.N.O.
6) Stitch top chords together with 4x4 20Ga plates at 24 in oc maximum, U.N.O.
7) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
8) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
9) A creep factor of 1.00 has been applied for this truss analysis.
10) ▭ Indicates non-structural members.
11) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 12, 13, 1 may need to be considered.
12) Listed wind uplift reactions based on MWFRS & C&C loading.

Habitat For Humanity
6564 State Hwy. 96
Olney Springs ,CO 81062
(719) 267-5323

Truss: J01
Job: PP - 3410_0 - B Risley4Bdr1S
Date: 03/17/20 07:45:23
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-11-11	5/12	2	1-0-0	0-0-0	0-0-0	0-0-0	1	24 in	10 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	30	Bldg Code:	IRC 2015/	TC:	0.21 (5-1)	Vert TL:	0 in	L / 999	(3-4)	L / 240
TCDL:	15		TPI 1-2014	BC:	0.02 (3-4)	Vert LL:	0 in	L / 999	(3-4)	L / 360
BCLL:	0	Rep Mbr:	No	Web:	0.03 (1-4)	Horz TL:	0 in		3	
BCDL:	10	Lumber D.O.L.:	115 %							

03/17/2020

COLORADO REGISTERED ARTURO A. HERNANDEZ 39632 PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	5.5 in	1.50 in	258 lbs	·	-41 lbs	-177 lbs	-177 lbs	91 lbs
3	1	1.5 in	---	106 lbs	-9 lbs	-46 lbs	-56 lbs	-56 lbs	·

Material

TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 4
Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (30 psf) roof snow loads. in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL= 1.15.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
5) Minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hanger is for graphical intrepretation only. Install hanger per manufacturer's recommendation.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) A creep factor of 1.00 has been applied for this truss analysis.
6) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 3 may need to be considered.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

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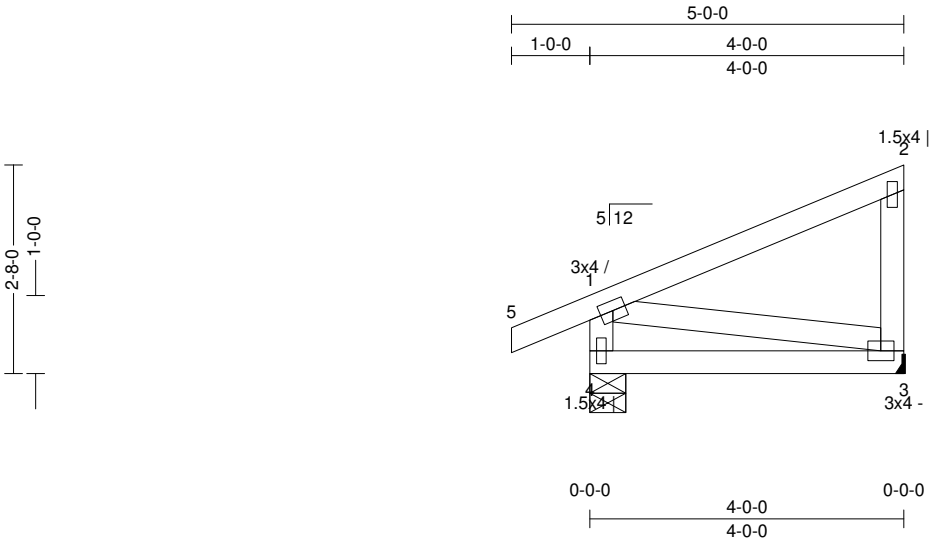
Eagle Metal Products

Habitat For Humanity
6564 State Hwy. 96
Olney Springs ,CO 81062
(719) 267-5323

Truss: J02
Job: PP - 3410_0 - B Risley4Bdr1S
Date: 03/17/20 07:45:28
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
4-0-0	5/12	4	1-0-0	0-0-0	0-0-0	0-0-0	1	24 in	17 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	30	Bldg Code :	IRC 2015/	TC :	0.21 (1-2)	Vert TL:	0.02 in	L / 999	(3-4)	L / 240
TCDL :	15		TPI 1-2014	BC :	0.12 (3-4)	Vert LL:	0.01 in	L / 999	(3-4)	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.08 (2-3)	Horz TL:	0 in		3	
BCDL :	10	Lumber D.O.L. :	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	5.5 in	1.50 in	366 lbs	·	-41 lbs	-240 lbs	-240 lbs	130 lbs
3	1	1.5 in	---	244 lbs	·	-73 lbs	-140 lbs	-140 lbs	·

03/17/2020

COLORADO REGISTERED
ARTURO A. HERNANDEZ
39632
PROFESSIONAL ENGINEER

Material

TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 4
Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (30 psf) roof snow loads. in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL= 1.15.
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5) Minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web				

Notes

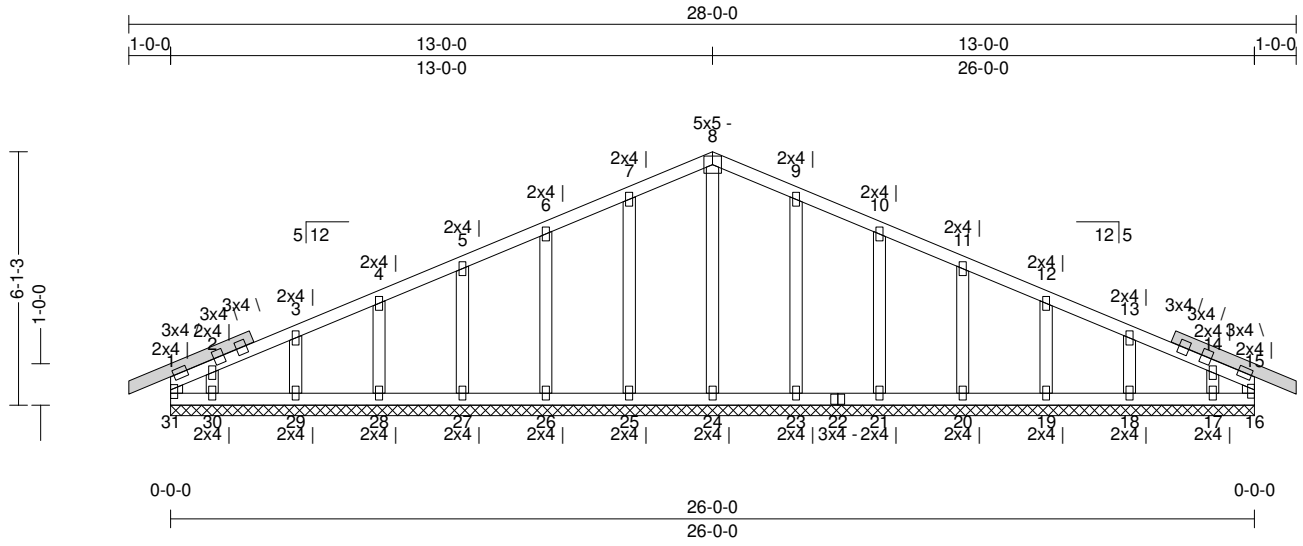
1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hanger is for graphical intrepretation only. Install hanger per manufacturer's recommendation.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) A creep factor of 1.00 has been applied for this truss analysis.
6) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild® Truss Software v5.6.355
Eagle Metal Products

Habitat For Humanity
6564 State Hwy. 96
Olney Springs ,CO 81062
(719) 267-5323

Truss: L01
Job: PP - 3410_0 - B Risley4Bdr1S
Date: 03/17/20 07:44:35
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-0-0	5/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	112 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflection		L/	(loc)	Allowed
TCLL : 30	Bldg Code :	IRC 2015/	TC : 0.21 (1-2)	Vert TL:	0 in UP	L / 999	16	L / 240
TCDL : 15		TPI 1-2014	BC : 0.02 (17-18)	Vert LL:	0 in	L / 999	16	L / 360
BCLL : 0	Rep Mbr :	No	Web : 0.10 (1-31)	Horz TL:	0 in			
BCDL : 10	Lumber D.O.L. :	115 %						

03/17/2020

COLORADO REGISTERED ARTURO A. HERNANDEZ 39632 PROFESSIONAL ENGINEER

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	306 lbs	139 plf	-5 lbs	-65 lbs	-86 lbs	-86 lbs	-121 lbs

Material

TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 4
Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL= 1.15.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL= 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC		
BC		
Web		

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 2x4 20ga plates, U.N.O.
5) Stitch top chords together with 20Ga plates at 24 in oc maximum, U.N.O.
6) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
7) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
8) A creep factor of 1.00 has been applied for this truss analysis.
9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 17, 30 may need to be considered.
10) Listed wind uplift reactions based on MWFRS & C&C loading.

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Habitat For Humanity
6564 State Hwy. 96
Olney Springs ,CO 81062
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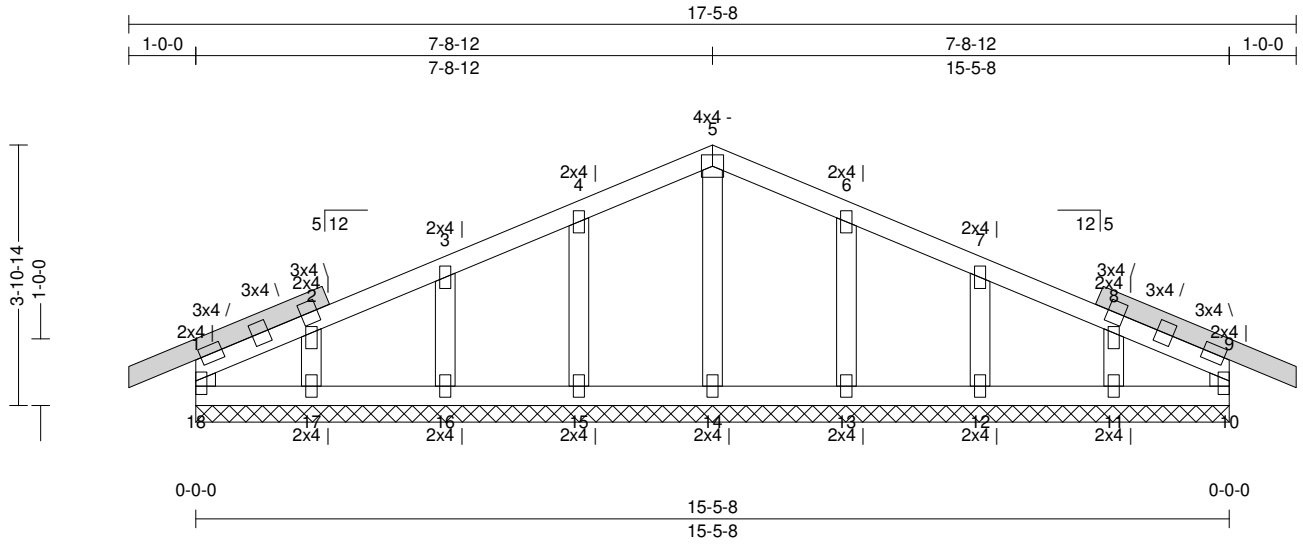
Truss: L02
Job: PP - 3410_0 - B Risley4Bdr1S
Date: 03/17/20 07:44:38
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
15-5-8	5/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	61 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	30	Bldg Code :	IRC 2015/ TPI 1-2014	TC :	0.21 (1-2)	Vert TL:	0 in UP	L / 999	10	L / 240
TCDL :	15	Rep Mbr :	No	BC :	0.02 (15-16)	Vert LL:	0 in	L / 999	10	L / 360
BCLL :	0	Lumber D.O.L. :	115 %	Web :	0.10 (1-18)	Horz TL:	0 in			
BCDL :	10									

03/17/2020

COLORADO REGISTERED ARTURO A. HERNANDEZ 39632 PROFESSIONAL ENGINEER

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	305 lbs	152 plf	-	-94 lbs	-175 lbs	-175 lbs	-114 lbs

Material

TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 4
Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL= 1.15.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Partial, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL= 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC		
BC		
Web		

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 2x4 20ga plates, U.N.O.
5) Stitch top chords together with 20Ga plates at 24 in oc maximum, U.N.O.
6) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
7) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
8) A creep factor of 1.00 has been applied for this truss analysis.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

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Habitat For Humanity
6564 State Hwy. 96
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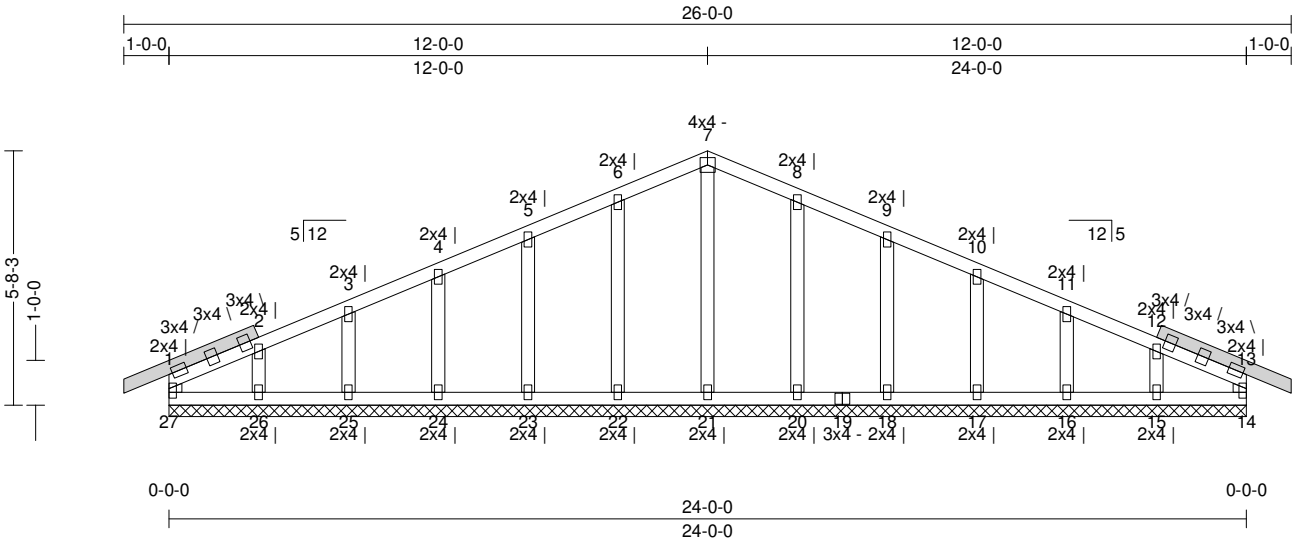
Truss: L03
Job: PP - 3410_0 - B Risley4Bdr1S
Date: 03/17/20 07:44:43
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-0-0	5/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	101 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL : 30	Bldg Code :	IRC 2015/	TC :	0.21 (1-2)	Vert TL:	0 in UP	L / 999	14	L / 240
TCDL : 15		TPI 1-2014	BC :	0.02 (14-15)	Vert LL:	0 in	L / 999	14	L / 360
BCLL : 0	Rep Mbr :	No	Web :	0.10 (1-27)	Horz TL:	0 in			
BCDL : 10	Lumber D.O.L. :	115 %							

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	305 lbs	138 plf	-	-65 lbs	-107 lbs	-107 lbs	-109 lbs

03/17/2020

COLORADO REGISTERED ARTURO A. HERNANDEZ 39632 PROFESSIONAL ENGINEER

Material
TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 4
Web: SPF 1650/1.5 2 x 4

Bracing
TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads
1) This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL= 1.15.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL= 1.60

Member Forces Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC		
BC		
Web		

Notes
1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 2x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
7) A creep factor of 1.00 has been applied for this truss analysis.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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6564 State Hwy. 96
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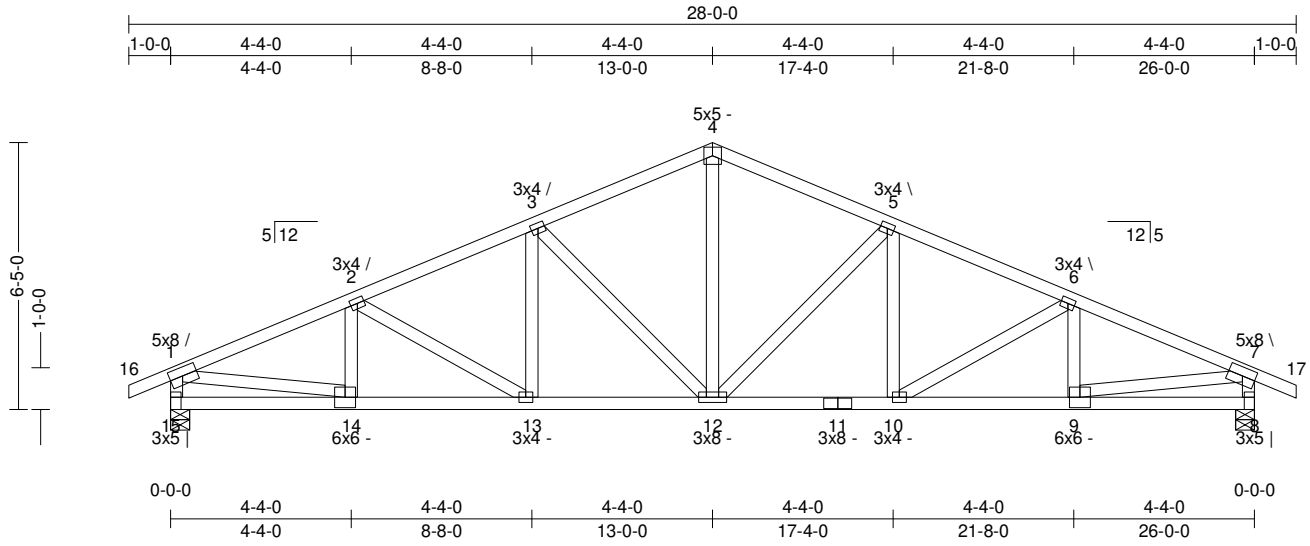
(719) 267-5323

Truss: T01
Job: PP - 3410_0 - B Risley4Bdr1S
Date: 03/17/20 07:44:49
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-0-0	5/12	12	1-0-0	1-0-0	0-0-0	0-0-0	1	24 in	119 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	30	Bldg Code:	IRC 2015/	TC:	0.32 (5-6)	Vert TL:	0.2 in	L / 999	(12-13)	L / 240
TCDL:	15		TPI 1-2014	BC:	0.45 (10-12)	Vert LL:	0.09 in	L / 999	12	L / 360
BCLL:	0	Rep Mbr:	Yes	Web:	0.40 (7-9)	Horz TL:	0.06 in		8	
BCDL:	10	Lumber D.O.L.:	115 %							

03/17/2020

COLORADO REGISTERED ARTURO A. HERNANDEZ 39632 PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	5.5 in	2.84 in	1,813 lbs	·	-298 lbs	-462 lbs	-462 lbs	-33 lbs
8	1	5.5 in	2.84 in	1,813 lbs	·	-298 lbs	-462 lbs	-462 lbs	·

Material

TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 4
Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 4-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL= 1.15.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
4) Minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.256	-2,712 lbs		3-4	0.304	-2,004 lbs		5-6	0.318	-2,562 lbs
	2-3	0.318	-2,562 lbs		4-5	0.304	-2,004 lbs		6-7	0.256	-2,712 lbs
BC	9-10	0.450	2,443 lbs	(-415 lbs)	12-13	0.453	2,291 lbs	(-333 lbs)			
	10-12	0.453	2,291 lbs	(-333 lbs)	13-14	0.450	2,443 lbs	(-415 lbs)			
Web	1-15	0.170	-1,734 lbs		5-12	0.358	-828 lbs				
	1-14	0.403	2,483 lbs	(-443 lbs)	7-9	0.403	2,483 lbs	(-443 lbs)			
	3-12	0.358	-828 lbs		7-8	0.170	-1,734 lbs				
	4-12	0.176	1,082 lbs	(-258 lbs)							

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 1.00 has been applied for this truss analysis.
5) Listed wind uplift reactions based on MWFRS & C&C loading.

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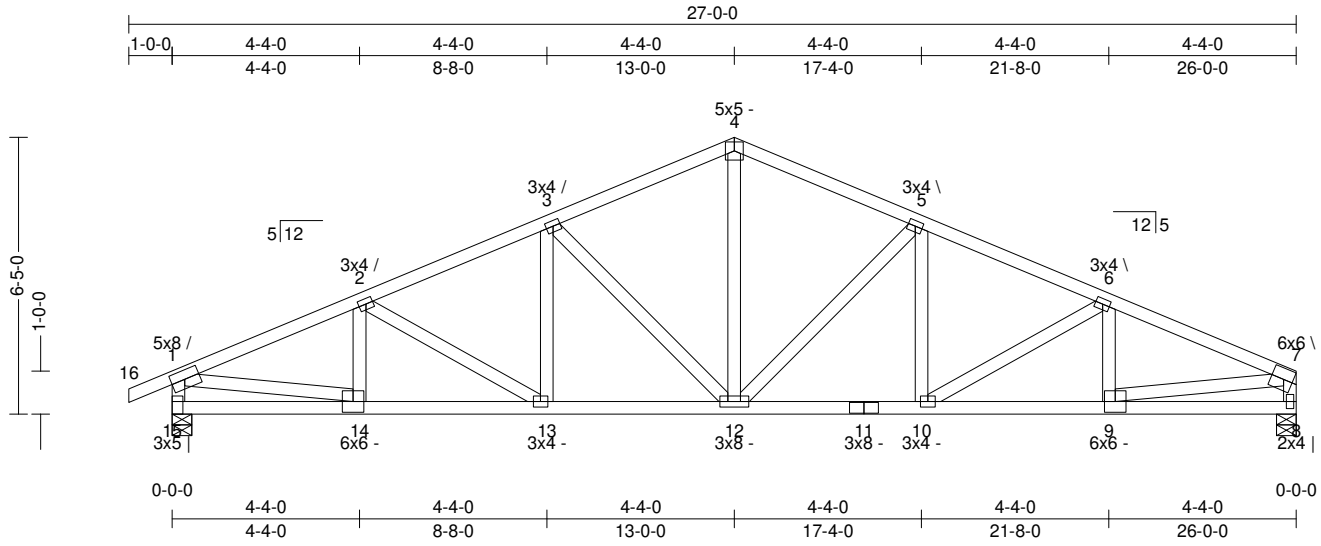
Eagle Metal Products

Habitat For Humanity
6564 State Hwy. 96
Olney Springs ,CO 81062
(719) 267-5323

Truss: T02
Job: PP - 3410_0 - B Risley4Bdr1S
Date: 03/17/20 07:44:52
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
26-0-0	5/12	12	1-0-0	0-0-0	0-0-0	0-0-0	1	24 in	117 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	30	Bldg Code :	IRC 2015/	TC :	0.32 (2-3)	Vert TL:	0.2 in	L / 999	(11-12)	L / 240
TCDL :	15		TPI 1-2014	BC :	0.45 (10-12)	Vert LL:	0.09 in	L / 999	(11-12)	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.41 (7-9)	Horz TL:	0.06 in		8	
BCDL :	10	Lumber D.O.L. :	115 %							

03/17/2020

COLORADO REGISTERED ARTURO A. HERNANDEZ 39632 PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	5.5 in	2.85 in	1,814 lbs	·	-298 lbs	-463 lbs	-463 lbs	53 lbs
8	1	5.5 in	2.70 in	1,721 lbs	·	-265 lbs	-378 lbs	-378 lbs	·

Material

TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 4
Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 4-1-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL= 1.15.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
4) Minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.256	-2,715 lbs		3-4	0.304	-2,009 lbs		5-6	0.314	-2,572 lbs
	2-3	0.318	-2,564 lbs		4-5	0.299	-2,009 lbs		6-7	0.296	-2,733 lbs
BC	9-10	0.454	2,468 lbs	(-474 lbs)	12-13	0.453	2,293 lbs	(-377 lbs)			
	10-12	0.454	2,299 lbs	(-379 lbs)	13-14	0.450	2,445 lbs	(-459 lbs)			
Web	1-15	0.170	-1,735 lbs		5-12	0.360	-832 lbs				
	1-14	0.403	2,485 lbs	(-445 lbs)	7-9	0.407	2,508 lbs	(-503 lbs)			
	3-12	0.358	-827 lbs		7-8	0.161	-1,641 lbs				
	4-12	0.176	1,085 lbs	(-259 lbs)							

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 1.00 has been applied for this truss analysis.
5) Listed wind uplift reactions based on MWFRS & C&C loading.

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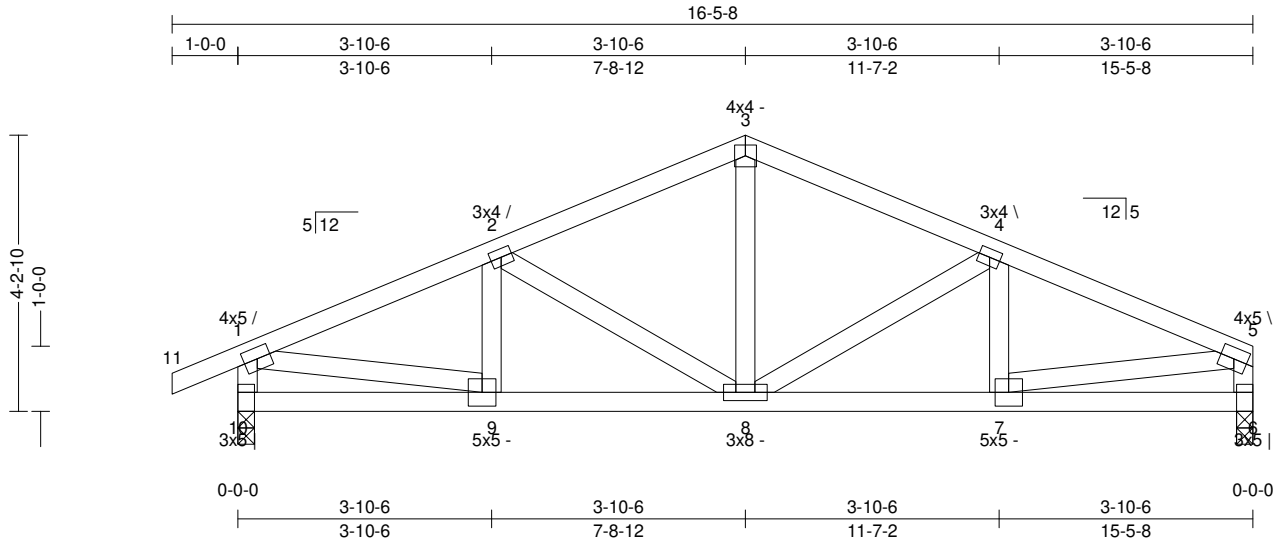
Habitat For Humanity
6564 State Hwy. 96
Olney Springs ,CO 81062
(719) 267-5323

Truss: T03
Job: PP - 3410_0 - B Risley4Bdr1S
Date: 03/17/20 07:44:56
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
15-5-8	5/12	3	1-0-0	0-0-0	0-0-0	0-0-0	1	24 in	65 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	30	Bldg Code :	IRC 2015/	TC :	0.26 (4-5)	Vert TL:	0.06 in	L / 999	(7-8)	L / 240
TCDL :	15		TPI 1-2014	BC :	0.29 (7-8)	Vert LL:	0.03 in UP	L / 999	(7-8)	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.23 (5-7)	Horz TL:	0.01 in		6	
BCDL :	10	Lumber D.O.L. :	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
10	1	3 in	1.90 in	1,211 lbs	·	-312 lbs	-627 lbs	-627 lbs	53 lbs
6	1	3 in	1.75 in	1,115 lbs	·	-264 lbs	-521 lbs	-521 lbs	·

Material
TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 4
Web: SPF 1650/1.5 2 x 4

Bracing
TC: Sheathed or Purlins at 5-7-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

03/17/2020
COLORADO REGISTERED ARTURO A. HERNANDEZ 39632 PROFESSIONAL ENGINEER

Loads
1) This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL= 1.15.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Partial, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
4) Minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.239	-1,557 lbs		3-4	0.254	-1,160 lbs				
	2-3	0.236	-1,155 lbs		4-5	0.258	-1,572 lbs				
BC	7-8	0.295	1,392 lbs	(-559 lbs)	8-9	0.291	1,371 lbs	(-542 lbs)			
Web	1-10	0.112	-1,140 lbs		3-8	0.076	471 lbs	(-220 lbs)	5-6	0.102	-1,044 lbs
	1-9	0.227	1,399 lbs	(-534 lbs)	4-8	0.116	-484 lbs				
	2-8	0.111	-462 lbs		5-7	0.231	1,421 lbs	(-602 lbs)			

Notes
1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 1.00 has been applied for this truss analysis.
5) Listed wind uplift reactions based on MWFRS & C&C loading.

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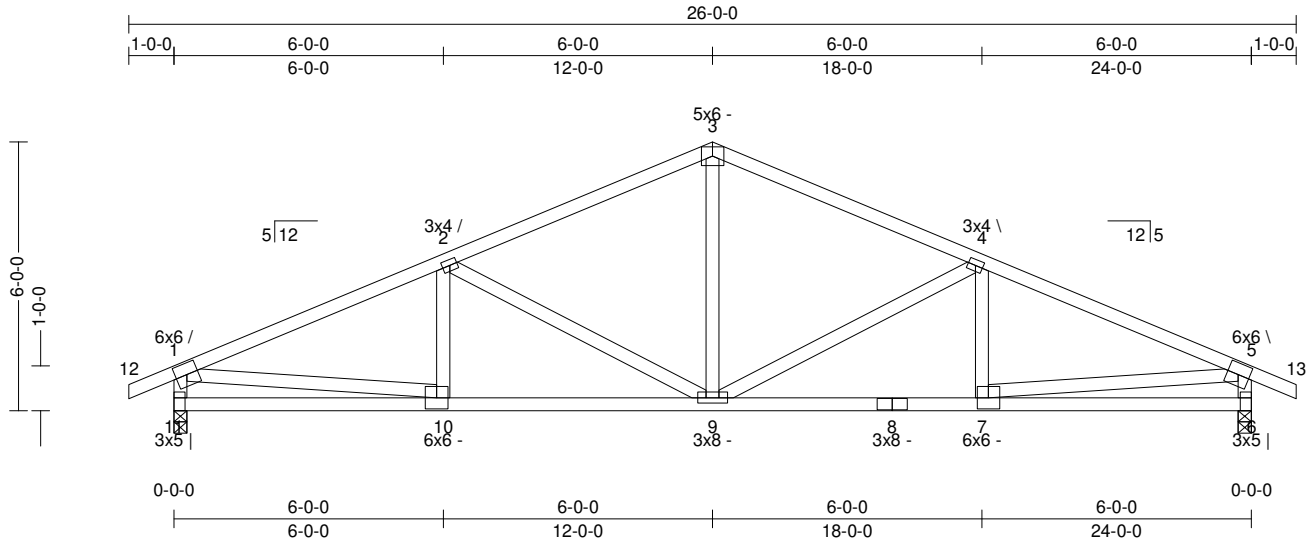
Eagle Metal Products

Habitat For Humanity
6564 State Hwy. 96
Olney Springs ,CO 81062
(719) 267-5323

Truss: T04
Job: PP - 3410_0 - B Risley4Bdr1S
Date: 03/17/20 07:45:00
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-0-0	5/12	6	1-0-0	1-0-0	0-0-0	0-0-0	1	24 in	101 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	30	Bldg Code :	IRC 2015/	TC :	0.57 (4-5)	Vert TL:	0.2 in	L / 999	(8-9)	L / 240
TCDL :	15		TPI 1-2014	BC :	0.56 (7-9)	Vert LL:	0.08 in	L / 999	(8-9)	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.45 (4-9)	Horz TL:	0.04 in		6	
BCDL :	10	Lumber D.O.L. :	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
11	1	3.5 in	2.66 in	1,697 lbs	·	-277 lbs	-433 lbs	-433 lbs	-33 lbs
6	1	3.5 in	2.66 in	1,697 lbs	·	-277 lbs	-433 lbs	-433 lbs	·

03/17/2020

COLORADO REGISTERED ARTURO A. HERNANDEZ 39632 PROFESSIONAL ENGINEER

Material
TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 4
Web: SPF 1650/1.5 2 x 4

Bracing
TC: Sheathed or Purlins at 4-1-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads
1) This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL= 1.15.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
4) Minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.565	-2,619 lbs		3-4	0.558	-1,903 lbs				
	2-3	0.558	-1,903 lbs		4-5	0.565	-2,619 lbs				
BC	7-9	0.556	2,342 lbs	(-366 lbs)	9-10	0.556	2,342 lbs	(-366 lbs)			
Web	1-11	0.156	-1,594 lbs		3-9	0.138	852 lbs	(-150 lbs)	5-6	0.156	-1,594 lbs
	1-10	0.384	2,362 lbs	(-391 lbs)	4-9	0.452	-842 lbs				
	2-9	0.452	-842 lbs		5-7	0.384	2,362 lbs	(-391 lbs)			

Notes
1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 1.00 has been applied for this truss analysis.
5) Listed wind uplift reactions based on MWFRS & C&C loading.

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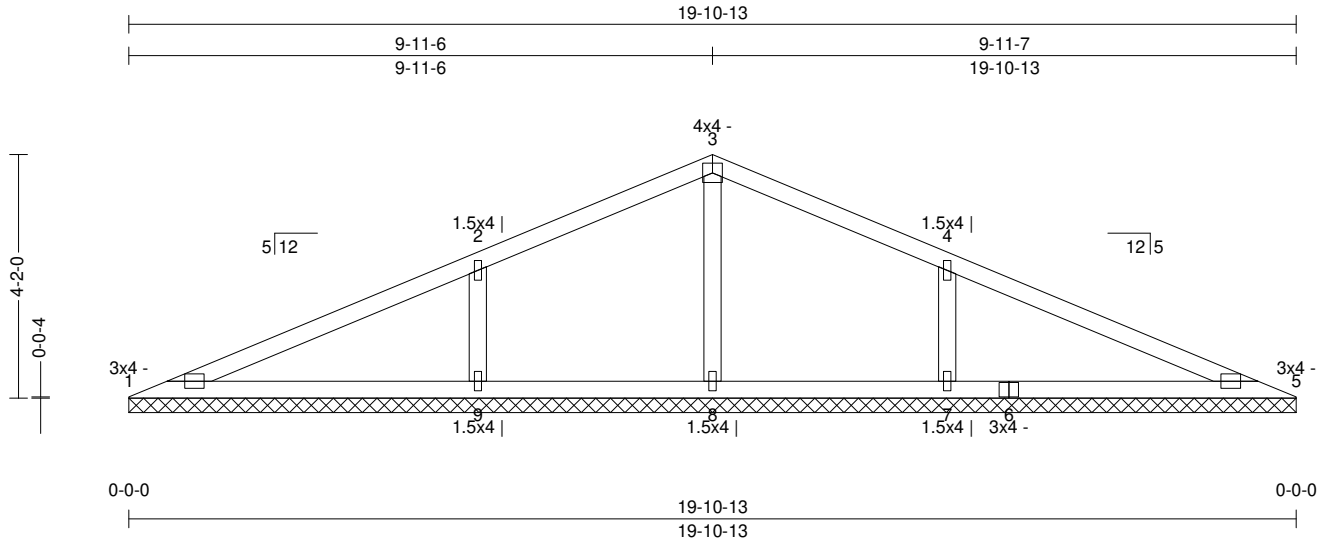
Eagle Metal Products

Habitat for Humanity of Colorado
P.O. Box 100
Onley Springs, CO 81062
(719) 267-5323

Truss: V01
Job: PP - 3410_0 - B Risley4Bdr1S
Date: 03/17/20 07:45:04
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
19-10-13	5/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	52 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	30	Bldg Code:	IRC 2015/ TPI 1-2014	TC:	0.36 (1-2)	Vert TL:	0.01 in	L / 999	(9-1)	L / 240
TCDL:	15	Rep Mbr:	No	BC:	0.17 (9-1)	Vert LL:	0 in	L / 999	(9-1)	L / 360
BCLL:	0	Lumber D.O.L.:	115 %	Web:	0.07 (2-9)	Horz TL:	0 in			
BCDL:	10									

03/17/2020

COLORADO REGISTERED ARTURO A. HERNANDEZ 39632 PROFESSIONAL ENGINEER

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	687 lbs	164 plf	-165 lbs	-160 lbs	-214 lbs	-214 lbs	345 lbs

Material

TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 4
Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL= 1.15.
2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL= 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC											
BC											
Web	2-9	0.065	-591 lbs		4-7	0.065	-590 lbs				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 48 " OC, U.N.O.
4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
5) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
6) A creep factor of 1.00 has been applied for this truss analysis.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 1, 5, 1, 5 may need to be considered.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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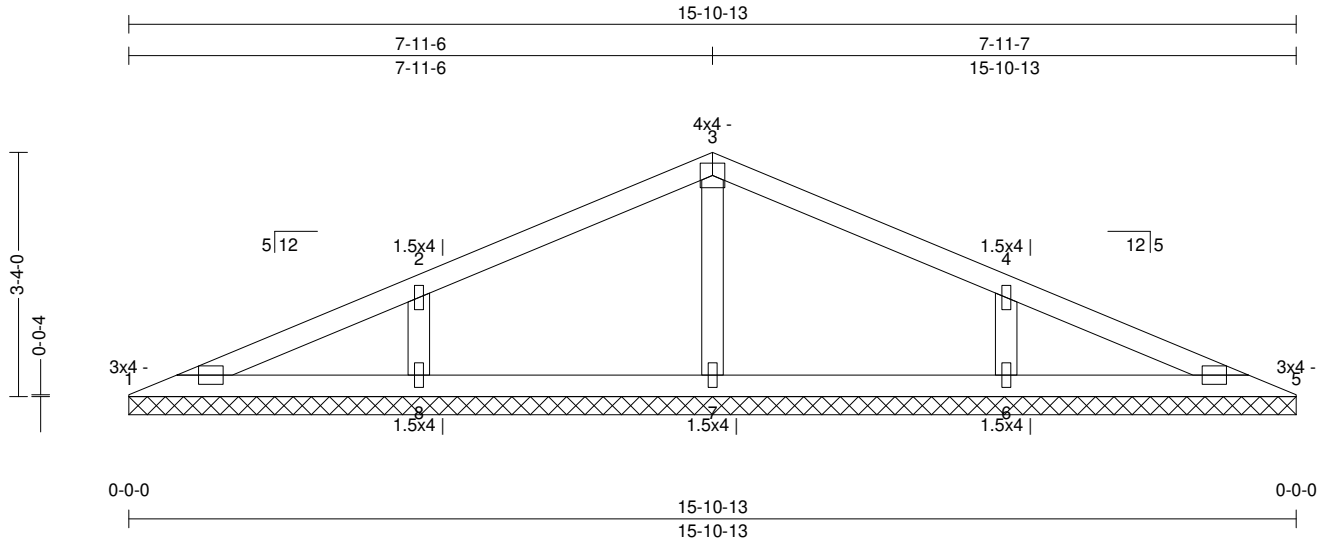
Truss: V02
Job: PP - 3410_0 - B Risley4Bdr1S
Date: 03/17/20 07:45:08
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
15-10-13	5/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	41 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	30	Bldg Code:	IRC 2015/ TPI 1-2014	TC:	0.25 (4-5)	Vert TL:	0 in	L / 999	(6-7)	L / 240
TCDL:	15	Rep Mbr:	No	BC:	0.08 (6-7)	Vert LL:	0 in	L / 999	5	L / 360
BCLL:	0	Lumber D.O.L.:	115 %	Web:	0.05 (4-6)	Horz TL:	0 in			
BCDL:	10									

03/17/2020

COLORADO REGISTERED ARTURO A. HERNANDEZ 39632 PROFESSIONAL ENGINEER

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	·	558 lbs	141 plf	-23 lbs	-105 lbs	-244 lbs	-244 lbs	140 lbs

Material

TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 4
Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL= 1.15.
2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL= 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC											
BC											
Web	2-8	0.049	-484 lbs	3-7	0.045	-325 lbs	4-6	0.049	-484 lbs		

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 48 " OC, U.N.O.
4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
5) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
6) A creep factor of 1.00 has been applied for this truss analysis.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 1, 5, 1, 5 may need to be considered.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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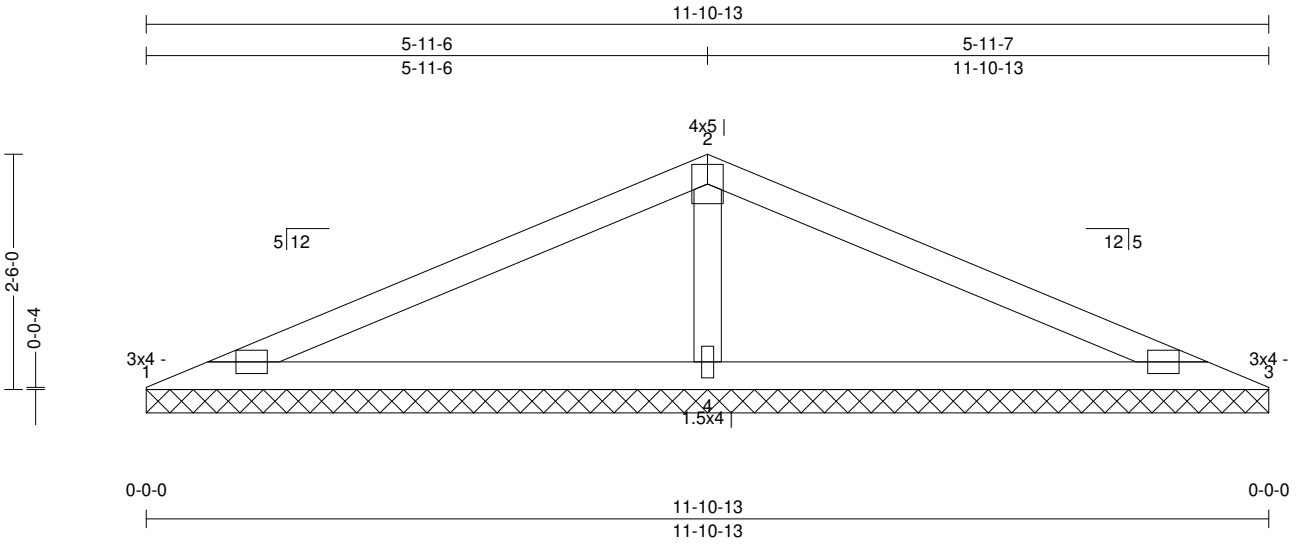
Truss: V03
Job: PP - 3410_0 - B Risley4Bdr1S
Date: 03/17/20 07:45:12
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
11-10-13	5/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	28 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	30	Bldg Code:	IRC 2015/ TPI 1-2014	TC:	0.48 (2-3)	Vert TL:	0.01 in	L / 999	(3-4)	L / 240
TCDL:	15	Rep Mbr:	No	BC:	0.21 (3-4)	Vert LL:	0.01 in	L / 999	(3-4)	L / 360
BCLL:	0	Lumber D.O.L.:	115 %	Web:	0.04 (2-4)	Horz TL:	0 in			
BCDL:	10									

~~03/17/2020~~

COLORADO REGISTERED ARTURO A. HERNANDEZ 39632 PROFESSIONAL ENGINEER

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	·	863 lbs	207 plf	-204 lbs	-152 lbs	-391 lbs	-391 lbs	459 lbs

Material

TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 4
Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL= 1.15.
2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL= 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.485	360 lbs	(-278 lbs)	2-3	0.485	360 lbs	(-278 lbs)			
BC											
Web	2-4	0.037	-335 lbs								

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 48 " OC, U.N.O.
4) Attach gable webs with 3x4 20ga plates, U.N.O.
5) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
6) A creep factor of 1.00 has been applied for this truss analysis.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 1, 3, 1, 3 may need to be considered.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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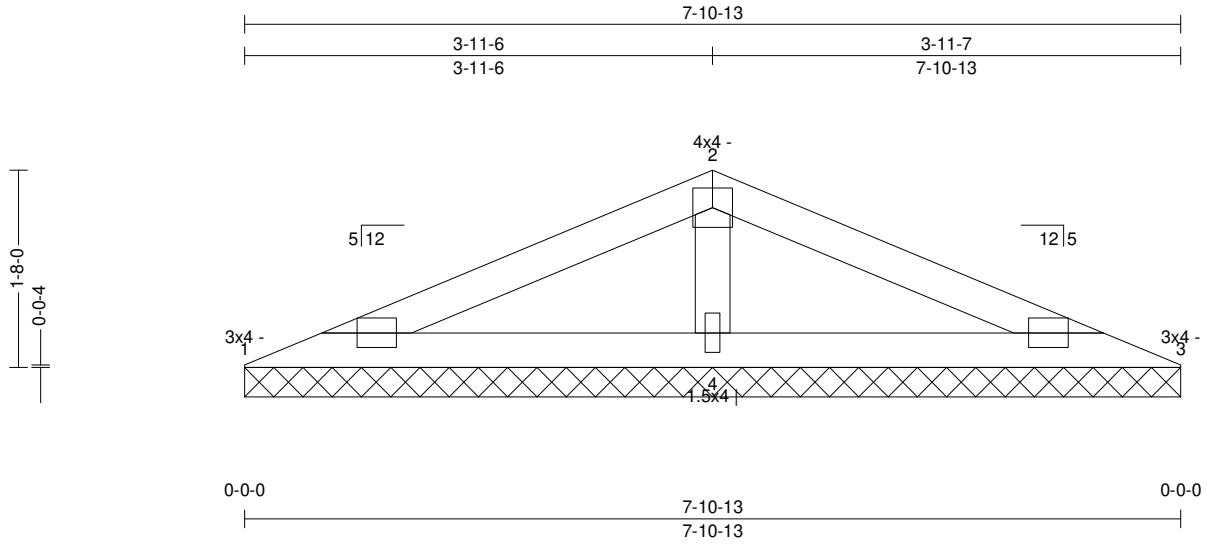
Truss: V04
Job: PP - 3410_0 - B Risley4Bdr1S
Date: 03/17/20 07:45:16
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
7-10-13	5/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	18 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	30	Bldg Code:	IRC 2015/ TPI 1-2014	TC:	0.16 (2-3)	Vert TL:	0 in	L / 999	(3-4)	L / 240
TCDL:	15	Rep Mbr:	No	BC:	0.06 (3-4)	Vert LL:	0 in	L / 999	3	L / 360
BCLL:	0	Lumber D.O.L.:	115 %	Web:	0.02 (2-4)	Horz TL:	0 in			
BCDL:	10									

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	414 lbs	153 plf	-49 lbs	-76 lbs	-219 lbs	-219 lbs	188 lbs

03/17/2020

COLORADO REGISTERED ARTURO A. HERNANDEZ 39632 PROFESSIONAL ENGINEER

Material

TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 4
Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL= 1.15.
2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL= 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC			
BC			
Web			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 48 "OC, U.N.O.
4) Attach gable webs with 3x4 20ga plates, U.N.O.
5) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
6) A creep factor of 1.00 has been applied for this truss analysis.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 1, 3, 1, 3 may need to be considered.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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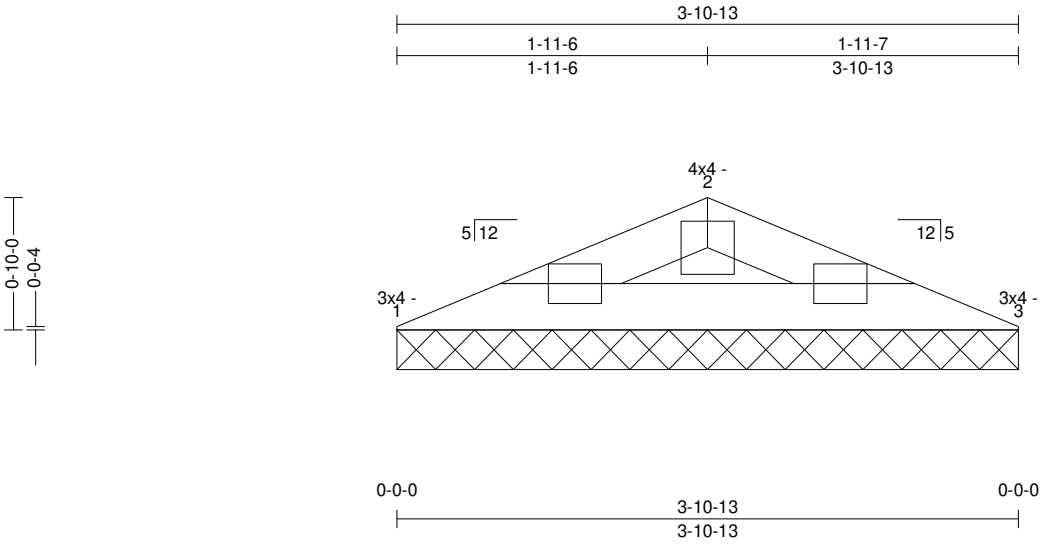
(719) 267-5323

Truss: V05
Job: PP - 3410_0 - B Risley4Bdr1S
Date: 03/17/20 07:45:20
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
3-10-13	5/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	8 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	30	Bldg Code:	IRC 2015/	TC:	0.02 (2-3)	Vert TL:	0 in	L / 999	3	L / 240
TCDL:	15		TPI 1-2014	BC:	0.01 (3-1)	Vert LL:	0 in	L / 999	3	L / 360
BCLL:	0	Rep Mbr:	No	Web:	0.00 (1)	Horz TL:	0 in			
BCDL:	10	Lumber D.O.L.:	115 %							

03/17/2020

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	155 lbs	110 plf	-	-34 lbs	-113 lbs	-113 lbs	-106 lbs

Material

TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 4
Web:

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (30 psf) roof snow loads. in accordance with ASCE7 - 10 except as noted, with the following user defined input: 30 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL= 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 48 " OC, U.N.O.
4) Attach gable webs with 3x4 20ga plates, U.N.O.
5) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
6) A creep factor of 1.00 has been applied for this truss analysis.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

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