

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Habitat for Humanity of Colorado**.

These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: **PP - 3410_0 - C Risley4Bdr1S - 1122759**

G01, G02, G03, G04, HDL01, HDL02, J01, J02, J03, J04, L01, L02, T01, T02, T03, T04, T05, T06, T07, T08, T09, T10, V01, V02, V03, V04, V05

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.



Arturo A. Hernandez (CO, PE-39632)

My license renewal date for the state of CO is 10/31/2021.

IMPORTANT NOTE: The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

DESIGN NOTES

1. The Truss Design Drawing(s) provided with these General Notes have been prepared under and are subject to ANSI/TPI1. Capitalized terms have the meanings provided in ANSI/TPI1.
2. Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.

DESIGN LIMITATIONS

3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANSI/TPI1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
6. Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

HANDLING, INSTALLING, & BRACING

7. Refer to BCSI for handling, installing, restraining and bracing trusses. Copies can be obtained from the Truss Plate Institute (TPI), 218 N Lee Street, Suite 312, Alexandria, VA 22314, www.tpinst.org or SBCA, 6300 Enterprise Lane, Madison, WI 53719, www.sbcindustry.com.
8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
9. Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
10. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.

11. Bottom chord required bracing shall be at 10ft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
12. Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
13. Never exceed the design loading shown and never stack building or other materials on inadequately braced truss; refer to BCSI.
14. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
15. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

MATERIALS & FABRICATION

16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
17. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
18. Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI/TPI1.
20. For a specified plate gauge and grade, the specified size is a minimum.
21. Connections not shown are the responsibility of others.
22. Adequate support shall be provided to resist gravity, lateral, uplift loads.
23. For 4X2 truss orientation, locate plates 0 - 1/16" from outside the edge of the truss.
24. Fabrication of truss shall be in accordance with ANSI/TPI1.

OTHER NOTES

25. Camber is a non-structural consideration and is the responsibility of truss fabricator.
26. Do not cut or alter any truss member or plate without prior approval from a professional engineer.
27. Lumber design values are in accordance with ANSI/TPI; lumber design values are by others.
28. Install specified hangers per manufacturer recommendations.

SYMBOLS

PLATE SIZE

3X4 - The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

-, /, |, Indicates required direction of slots; Reference "Joint Details" for more information.

20 Ga Gr40 connectors required
3X10-20HS - 20 Ga Gr60 connectors required
8X10-18HS - 18 Ga Gr60 connectors required

LATERAL BRACING

When this symbol shown, continuous lateral bracing is required on the web of the truss.



BEARING

Indicates location where bearings (supports) occur.



PLATE LOCATION & ORIENTATION

The plate shall be centered on joint and/or placed in accordance with the design drawing/QC full scale details.



REFERENCES

- ANSI/TPI1:** National Design Standard for Metal Plate Connected Wood Trusses
- BCSI:** Building Component & Safety Information - Guide to Good Practice for Handling, Installing, Restraining, & Bracing of Metal Plate Connected Wood Trusses.
- NDS:** National Design Specification for Wood Construction
- ESR:** 1082 published by the International Code Council. www.icc-es.org

Habitat for Humanity of Colorado

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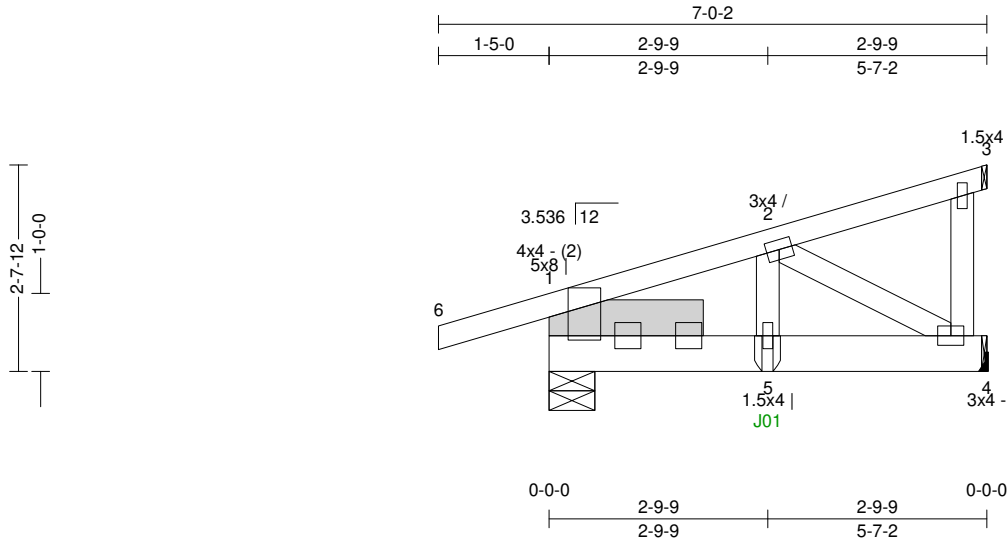
Truss: G01

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:43

Page: 1 of 2

SPAN 5-7-2	PITCH 3.536/12	QTY 1	OHL 1-5-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 29 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IRC 2015/	TC: 0.22 (6-1)	Vert TL: 0.01 in	L/999	(4-5)	L/240
TCLL: 30	TP1 1-2014	BC: 0.08 (4-5)	Vert LL: 0 in	L/999	(4-5)	L/360
TCDL: 15	Rep Mbr: No	Web: 0.08 (3-4)	Horz TL: 0 in		4	
BCLL: 0	Lumber D.O.L.: 115 %					
BCDL: 10						

03/17/2020

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	7.028 in	1.50 in	469 lbs	-	-194 lbs	-397 lbs	-397 lbs	135 lbs
4	1	1.5 in	--	341 lbs	-	-209 lbs	-241 lbs	-241 lbs	-

Material

TC: SPF 1650/1.5 2 x 4
 BC: SPF 1650/1.5 2 x 6
 Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



Loads

- This truss has been designed for the effects of balanced (30 psf) roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/CtCs = 1, DOL = 1.15.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Minimum storage attic loading has been applied in accordance with IRC 301.5

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-1-5-0	-0-4-6	Down	Proj	0 plf	32.48 plf	
Top	-0-4-6	2-10-4	Down	Proj	32.48 plf	0 plf	
Top	-1-5-0	-0-4-6	Down	Proj	0 plf	32.48 plf	
Top	-0-4-6	2-10-4	Down	Proj	32.48 plf	0 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-1-5-0	-0-4-6	Down	Proj	0 plf	16.24 plf	
Top	-0-4-6	2-10-4	Down	Proj	16.24 plf	0 plf	
Top	-1-5-0	-0-4-6	Down	Proj	0 plf	16.24 plf	
Top	-0-4-6	2-10-4	Down	Proj	16.24 plf	0 plf	

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.213	-393 lbs					
BC	4-5	0.079	353 lbs	(-210 lbs)	5-1	0.078	353 lbs	(-210 lbs)
Web	2-4	0.051	-408 lbs					

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

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 Eagle Metal Products

Habitat for Humanity of Colorado

P.O. Box 100
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Truss: G01

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:43

Page: 2 of 2

SPAN 5-7-2	PITCH 3.536/12	QTY 1	OHL 1-5-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 29 lbs
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Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
J01	BC	2-9-8
J01	BC	2-9-8

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.00 has been applied for this truss analysis.
- 6) Indicates non-structural members.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

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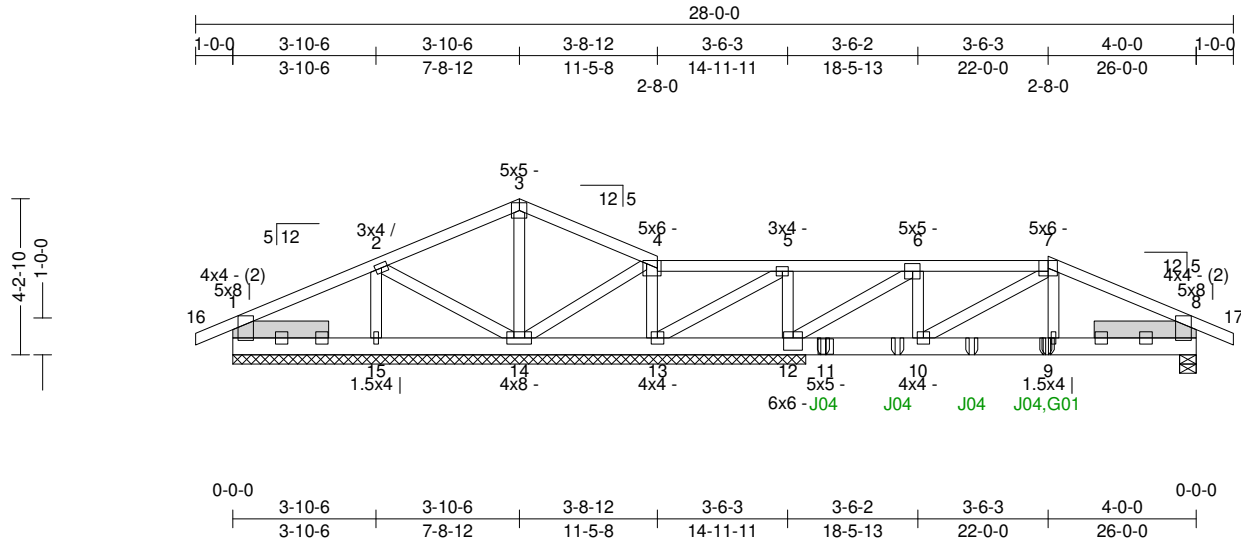
Truss: G02

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:49

Page: 1 of 2

SPAN 26-0-0	PITCH 5/12	QTY 1	OHL 1-0-0	OHR 1-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 127 lbs
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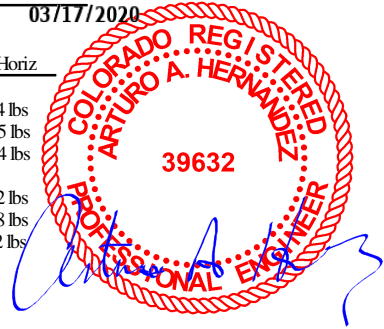
All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IRC 2015/	TC: 0.33 (4-5)	Vert TL: 0.06 in	L / 999	(9-10)	L / 240
TCLL: 30	TP1 1-2014	BC: 0.33 (9-10)	Vert LL: 0.03 in	L / 999	(9-10)	L / 360
TCDL: 15	Rep Mbr: No	Web: 0.40 (6-12)	Horz TL: 0.01 in		8	
BCLL: 0	Lumber D.O.L.: 115 %					
BCDL: 10						

03/17/2020

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
8	1	5.5 in	1.82 in	1,162 lbs	-	-373 lbs	-159 lbs	-373 lbs	-
12	1	185.5 in	N/A	1,941 lbs	-	-635 lbs	-56 lbs	-635 lbs	624 lbs
13	1	185.5 in	N/A	413 lbs	-	-52 lbs	-145 lbs	-145 lbs	-305 lbs
14	1	185.5 in	N/A	478 lbs	-	-150 lbs	-184 lbs	-184 lbs	-314 lbs
15	1	185.5 in	N/A	453 lbs	-	-77 lbs	-122 lbs	-122 lbs	-
1	1	185.5 in	N/A	207 lbs	-8 lbs	-30 lbs	-72 lbs	-72 lbs	142 lbs
1	1	185.5 in	N/A	255 lbs	-7 lbs	-95 lbs	-90 lbs	-95 lbs	138 lbs
1	1	185.5 in	N/A	59 lbs	-	-17 lbs	-9 lbs	-17 lbs	52 lbs



Material

TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 6
Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 5-6-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/CtCs = 1, DOL = 1.15.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Minimum storage attic loading has been applied in accordance with IRC 301.5

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-1-0-0	27-0-0	Down	Proj	30 plf	30 plf	
Top	23-0-0	27-0-0	Down	Proj	30 plf	30 plf	
Top	-1-0-0	14-11-4	Down	Proj	8.44 plf	8.44 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-1-0-0	27-0-0	Down	Proj	15 plf	15 plf	
Top	23-0-0	27-0-0	Down	Proj	15 plf	15 plf	
Top	-1-0-0	14-11-4	Down	Proj	4.22 plf	4.22 plf	
Bot	0-0-0	26-0-0	Down	Proj	10 plf	10 plf	
Bot	23-0-0	26-0-0	Down	Proj	10 plf	10 plf	
Bot	0-0-0	14-11-4	Down	Proj	2.81 plf	2.81 plf	

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

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P.O. Box 100
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Truss: G02

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:49

Page: 2 of 2

SPAN 26-0-0	PITCH 5/12	QTY 1	OHL 1-0-0	OHR 1-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 127 lbs
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Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	5-6	0.292	624 lbs	(-197 lbs)								
	6-7	0.136	-1,075 lbs									
	7-8	0.207	-1,589 lbs									
BC	8-9	0.257	1,409 lbs	(-429 lbs)	10-12	0.268	1,075 lbs	(-273 lbs)				
	9-10	0.330	1,434 lbs	(-438 lbs)								
Web	2-15	0.040	-363 lbs		5-13	0.070	-430 lbs	(-132 lbs)	6-10	0.127	785 lbs	(-219 lbs)
	3-14	0.073	-404 lbs		5-12	0.053	-490 lbs		7-10	0.088	-415 lbs	
	4-13	0.057	-508 lbs		6-12	0.403	-1,999 lbs		7-9	0.086	529 lbs	(-183 lbs)

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
J04	BC	15-11-4
J04	BC	17-11-4
J04	BC	19-11-4
J04	BC	21-11-4
G01	BC	22-0-0

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.00 has been applied for this truss analysis.
- 6) Indicates non-structural members.
- 7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 1, 1 may need to be considered.
- 8) Listed wind uplift reactions based on MWFRS & C&C loading.

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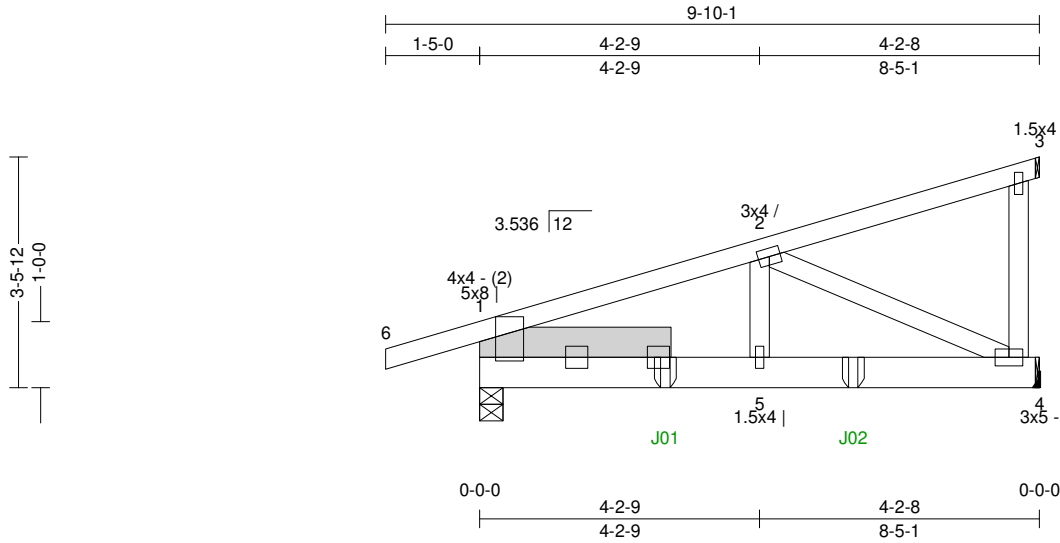
Truss: G03

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:46

Page: 1 of 2

SPAN 8-5-1	PITCH 3.536/12	QTY 2	OHL 1-5-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 41 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IRC 2015/	TC: 0.24 (1-2)	Vert TL: 0.04 in	L/999	(4-5)	L/240
TCLL: 30	TPH 1-2014	BC: 0.36 (4-5)	Vert LL: 0.02 in	L/999	(4-5)	L/360
TCDL: 15	Rep Mbr: No	Web: 0.23 (2-4)	Horz TL: 0.01 in		4	
BCLL: 0	Lumber D.O.L.: 115 %					
BCDL: 10						

Reaction

03/17/2020

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	4.2 in	1.50 in	775 lbs		-319 lbs	-427 lbs	-427 lbs	177 lbs
4	1	1.5 in	--	769 lbs		-378 lbs	-294 lbs	-378 lbs	

Material

TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 6
Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



Loads

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/Cts = 1, DOL = 1.15.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Minimum storage attic loading has been applied in accordance with IRC 301.5

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-1-5-0	-0-4-6	Down	Proj	0 plf	32.48 plf	
Top	-0-4-6	2-10-4	Down	Proj	32.48 plf	0 plf	
Top	-1-5-0	-0-4-6	Down	Proj	0 plf	32.48 plf	
Top	-0-4-6	2-10-4	Down	Proj	32.48 plf	0 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-1-5-0	-0-4-6	Down	Proj	0 plf	16.24 plf	
Top	-0-4-6	2-10-4	Down	Proj	16.24 plf	0 plf	
Top	-1-5-0	-0-4-6	Down	Proj	0 plf	16.24 plf	
Top	-0-4-6	2-10-4	Down	Proj	16.24 plf	0 plf	

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

Member	1/2	0.244	-1,024 lbs				
TC	4/5	0.361	946 lbs	(-468 lbs)	5-1	0.197	946 lbs (-468 lbs)
BC	2/5	0.086	531 lbs	(-131 lbs)	2/4	0.231	-1,048 lbs
Web							

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P.O. Box 100
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(719) 267-5323

Truss: G03

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:46

Page: 2 of 2

SPAN 8-5-1	PITCH 3.536/12	QTY 2	OHL 1-5-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 41 lbs
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Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
J01	BC	2-9-8
J01	BC	2-9-8
J02	BC	5-7-7
J02	BC	5-7-7

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.00 has been applied for this truss analysis.
- 6) Indicates non-structural members.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

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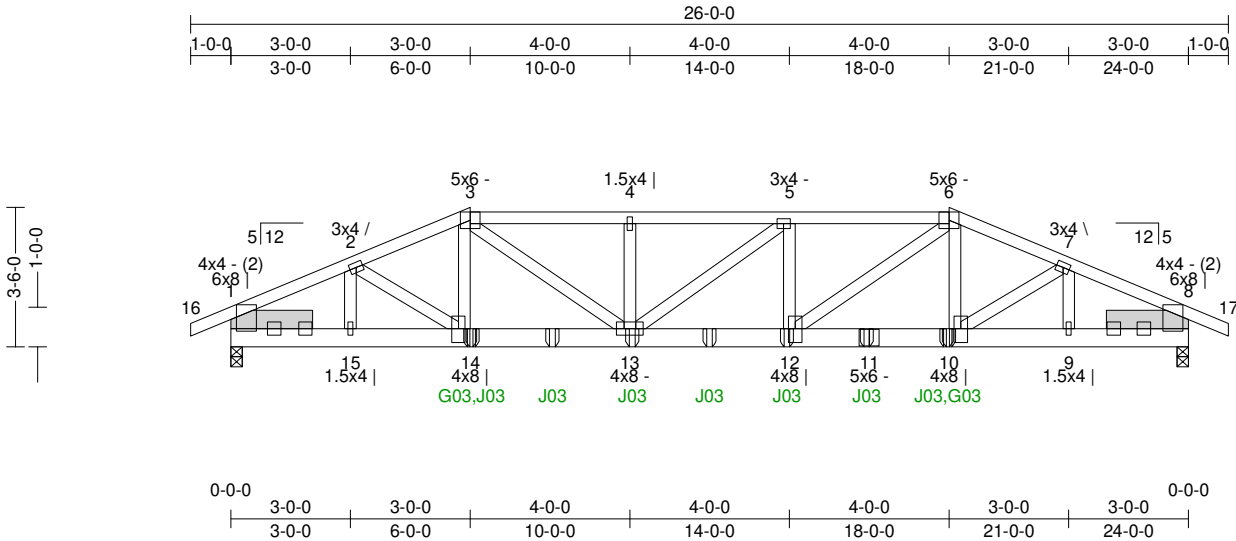
Truss: G04

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:52

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SPAN 24-0-0	PITCH 5/12	QTY 1	OHL 1-0-0	OHR 1-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 2	SPACING 24 in	WGT/PLY 122 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IRC 2015/	TC: 0.39 (5-6)	Vert TL: 0.28 in	L/990	(12-13)	L/240
TCLL: 30	TP1 1-2014	BC: 0.53 (12-13)	Vert LL: 0.14 in	L/999	(12-13)	L/360
TCDL: 15	Rep Mbr: No	Web: 0.17 (3-13)	Horz TL: 0.06 in		8	
BCLL: 0	Lumber D.O.L.: 115 %					
BCDL: 10						

03/17/2020

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	2.80 in	3,568 lbs		-1,121 lbs	-64 lbs	-1,121 lbs	
8	1	3.5 in	2.80 in	3,568 lbs		-1,121 lbs	-64 lbs	-1,121 lbs	

Material

TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 6
Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/Cts = 1, DOL = 1.15.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Minimum storage attic loading has been applied in accordance with IRC 301.5

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-1-0-0	25-0-0	Down	Proj	29.06 plf	29.06 plf	
Top	-1-0-0	5-0-0	Down	Proj	30.94 plf	30.94 plf	
Top	19-0-0	25-0-0	Down	Proj	30.94 plf	30.94 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-1-0-0	25-0-0	Down	Proj	14.53 plf	14.53 plf	
Top	-1-0-0	5-0-0	Down	Proj	15.47 plf	15.47 plf	
Top	19-0-0	25-0-0	Down	Proj	15.47 plf	15.47 plf	
Bot	0-0-0	24-0-0	Down	Proj	9.69 plf	9.69 plf	
Bot	0-0-0	5-0-0	Down	Proj	10.31 plf	10.31 plf	
Bot	19-0-0	24-0-0	Down	Proj	10.31 plf	10.31 plf	

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.268	-2,694 lbs	4-5	0.390	-3,857 lbs	7-8	0.298	-2,695 lbs
	2-3	0.325	-3,277 lbs	5-6	0.394	-3,868 lbs			
	3-4	0.393	-3,857 lbs	6-7	0.325	-3,274 lbs			
BC	8-9	0.521	2,446 lbs (-751 lbs)	10-12	0.429	3,029 lbs (-954 lbs)	13-14	0.438	3,032 lbs (-955 lbs)
	9-10	0.509	2,446 lbs (-751 lbs)	12-13	0.530	3,868 lbs (-1,203 lbs)	14-15	0.510	2,445 lbs (-751 lbs)
Web	2-15	0.063	-635 lbs	3-13	0.167	1,028 lbs (-298 lbs)	7-10	0.110	677 lbs (-235 lbs)
	2-14	0.111	681 lbs (-236 lbs)	6-12	0.167	1,028 lbs (-305 lbs)	7-9	0.063	-632 lbs
	3-14	0.080	494 lbs (-167 lbs)	6-10	0.079	486 lbs (-164 lbs)			



ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild@Truss Software V5.6.355
Eagle Metal Products

Habitat for Humanity of Colorado

P.O. Box 100
 Onley Springs, CO 81062
 (719) 267-5323

Truss: G04

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:52

Page: 2 of 2

SPAN 24-0-0	PITCH 5/12	QTY 1	OHL 1-0-0	OHR 1-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 2	SPACING 24 in	WGT/PLY 122 lbs
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Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
G03	BC	6-0-0
J03	BC	6-0-12
J03	BC	8-0-12
J03	BC	10-0-12
J03	BC	12-0-0
J03	BC	13-11-4
J03	BC	15-11-4
J03	BC	17-11-4
G03	BC	18-0-0

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.00 has been applied for this truss analysis.
- 6) Indicates non-structural members.
- 7) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply: 12d Nails or Gun Nails [min .135"x3] TC - 1 row @ 12 in oc, BC - 2 staggered rows @ 12 in oc, Webs - 1 row @ 12 in oc.

Provided the hanger connections do not adequately transfer the applied load to all plies: in addition to connectors shown above, attach each pair of girder plies with supplemental 12d Nails or Gun Nails [min .135"x3] as follows within 24" of the location shown:

BC: 6-0-0,(5)Connectors
 BC: 18-0-0,(5)Connectors

Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations.

- 8) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.
- 9) Lateral bracing shall be attached to each ply.
- 10) All fasteners minimum 2-1/2" long, unless otherwise noted.
- 11) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.
- 12) Listed wind uplift reactions based on MWFRS & C&C loading.

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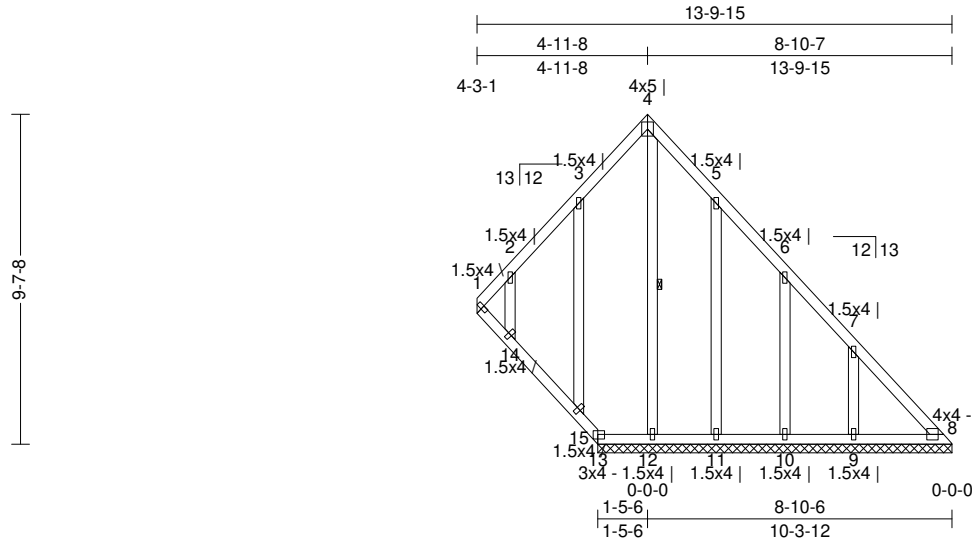
Truss: HDL01

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:44:48

Page: 1 of 1

SPAN 8-10-7	PITCH 13/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 74 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30 TCDL: 15 BCLL: 0 BCDL: 10	Bldg Code: IRC 2015/ TPI 1-2014 Rep Mbr: No Lumber D.O.L.: 115 %	TC: 0.27 (2-3) BC: 0.04 (8-9) Web: 0.39 (14-15)	Vert TL: 0 in Vert LL: 0 in Horz TL: 0 in	L / 999 L / 999	(8-9) 8	L / 240 L / 360

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		531 lbs	203 plf	-79 lbs			-79 lbs	-109 lbs

Material

TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 4
Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 4-12

Loads

1) This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL = 1.15.

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

Member	Member ID	Max CSI	Max Axial Force	Max Compr. Force
TC	2-3	0.274	321 lbs	
BC				
Web	3-15	0.165	-349 lbs	
	15-13	0.385	-382 lbs	
	4-12	0.131	-494 lbs	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable webs placed at 24" OC, U.N.O.
- 3) Attach gable webs with 1.5x4 20ga plates, U.N.O.
- 4) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 5) The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- 6) Gable must be sheathed on one side or lateral bracing applied appropriately.
- 7) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 8) A creep factor of 1.00 has been applied for this truss analysis.
- 9) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 10) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 8, 8 may need to be considered.

03/17/2020



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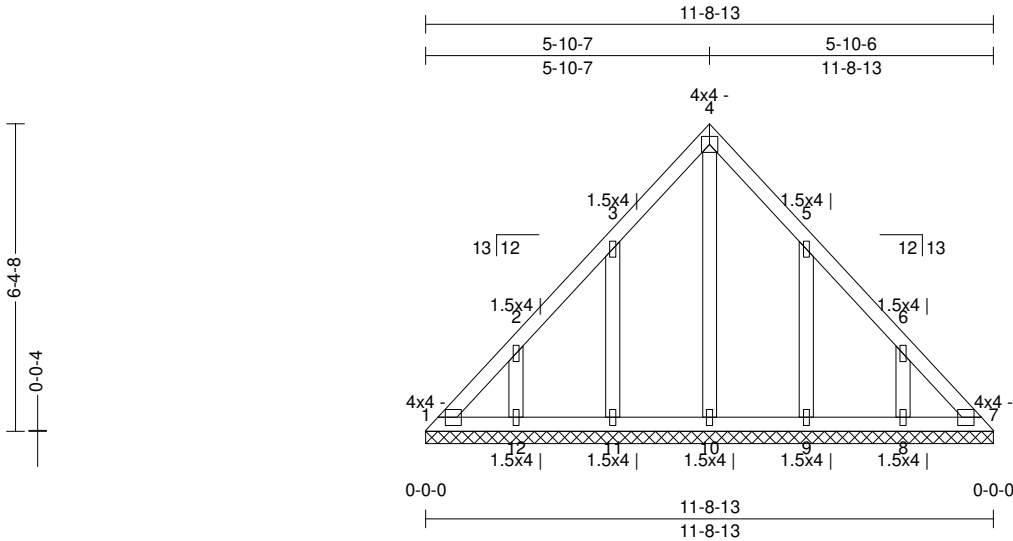
Truss: HDL02

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:44:51

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SPAN 11-8-13	PITCH 13/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 49 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IRC 2015/	TC: 0.10 (2-3)	Vert TL: 0 in	L/999	7	L/240
TCDL: 15	TPI 1-2014	BC: 0.02 (10-11)	Vert LL: 0 in	L/999	7	L/360
BCLL: 0	Rep Mbr: No	Web: 0.06 (5-9)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		365 lbs	149 plf	-0 lbs			-0 lbs	56 lbs

Material

TC: SPF 1650/1.5 2 x 4
 BC: SPF 1650/1.5 2 x 4
 Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL = 1.15.

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

Member	Max CSI	Max Axial Force
TC		
BC		
Web	3-11 0.060 -324 lbs	
	5-9 0.060 -324 lbs	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 24" OC, U.N.O.
- 4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- 7) A creep factor of 1.00 has been applied for this truss analysis.



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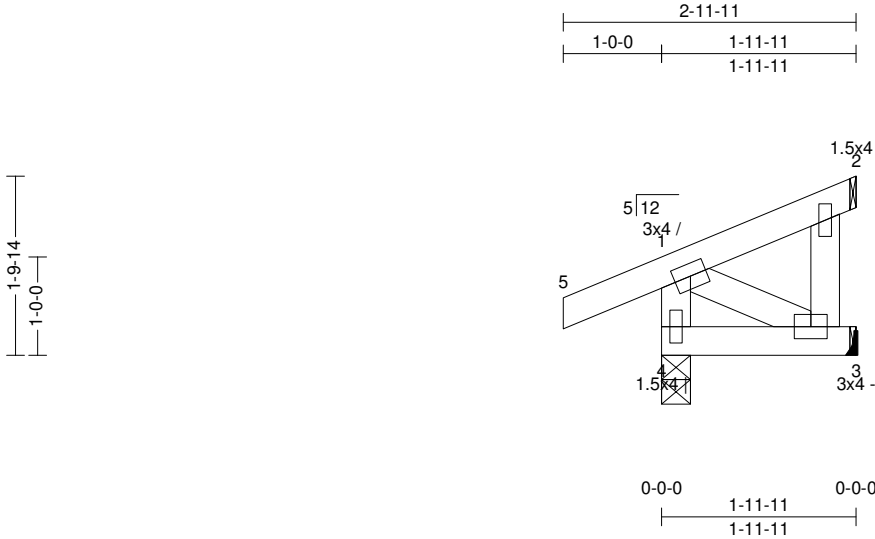
Truss: J01

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:34

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SPAN 1-11-11	PITCH 5/12	QTY 6	OHL 1-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 10 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30 TCDL: 15 BCLL: 0 BCDL: 10	Bldg Code: IRC 2015/ TP1 1-2014 Rep Mbr: Yes Lumber D.O.L.: 115 %	TC: 0.18 (5-1) BC: 0.02 (3-4) Web: 0.03 (1-4)	Vert TL: 0 in Vert LL: 0 in Horz TL: 0 in	L / 999 L / 999	(3-4) (3-4) 3	L / 240 L / 360

Reaction

03/17/2020

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	3.5 in	1.50 in	258 lbs	-	-41 lbs	-177 lbs	-177 lbs	91 lbs
3	1	1.5 in	--	106 lbs	-9 lbs	-46 lbs	-56 lbs	-56 lbs	-

Material

TC: SPF 1650/1.5 2 x 4
 BC: SPF 1650/1.5 2 x 4
 Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



Loads

- 1) This truss has been designed for the effects of balanced (30 psf) roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/CtCs = 1, DOL = 1.15.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.00 has been applied for this truss analysis.
- 6) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 3 may need to be considered.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

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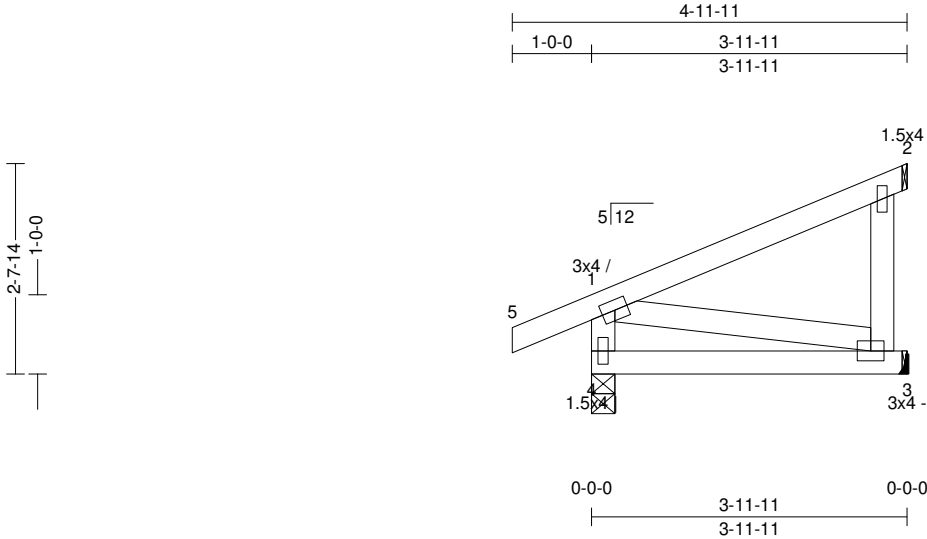
Truss: J02

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:36

Page: 1 of 1

SPAN 3-11-11	PITCH 5/12	QTY 4	OHL 1-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 17 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30 TCDL: 15 BCLL: 0 BCDL: 10	Bldg Code: IRC 2015/ TP1 1-2014 Rep Mbr: Yes Lumber D.O.L.: 115 %	TC: 0.18 (5-1) BC: 0.11 (3-4) Web: 0.08 (2-3)	Vert TL: 0.02 in Vert LL: 0.01 in Horz TL: 0 in	L/999 L/999	(3-4) (3-4) 3	L/240 L/360

Reaction

03/17/2020

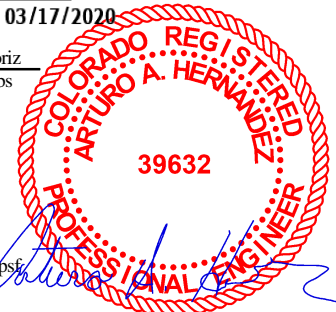
JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	3.5 in	1.50 in	353 lbs	-	-38 lbs	-231 lbs	-231 lbs	130 lbs
3	1	1.5 in	--	254 lbs	-	-76 lbs	-147 lbs	-147 lbs	-

Material

TC: SPF 1650/1.5 2 x 4
 BC: SPF 1650/1.5 2 x 4
 Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



Loads

- 1) This truss has been designed for the effects of balanced (30 psf) roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf, Roof (GSL = 30 psf), Ce/CtCs = 1, DOL = 1.15.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.00 has been applied for this truss analysis.
- 6) Listed wind uplift reactions based on MWFRS & C&C loading.

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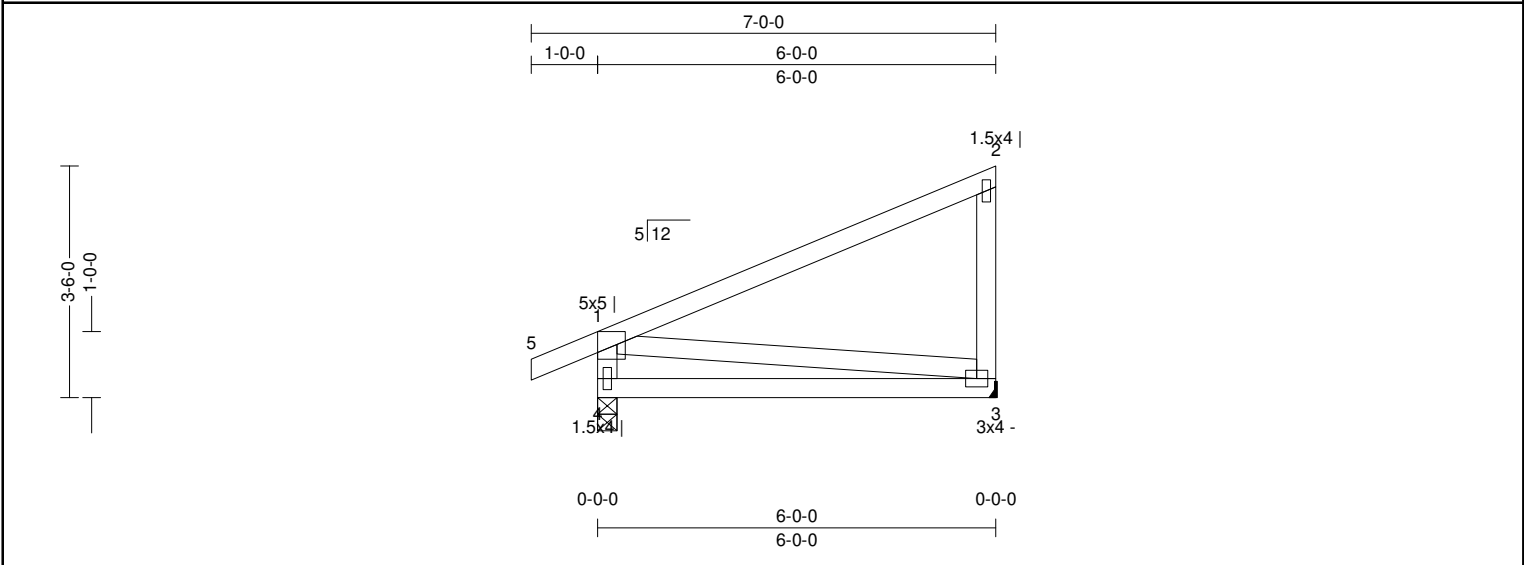
Truss: J03

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:38

Page: 1 of 1

SPAN 6-0-0	PITCH 5/12	QTY 7	OHL 1-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 25 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30 TCDL: 15 BCLL: 0 BCDL: 10	Bldg Code: IRC 2015/ TP1 1-2014 Rep Mbr: Yes Lumber D.O.L.: 115 %	TC: 0.61 (1-2) BC: 0.29 (3-4) Web: 0.14 (2-3)	Vert TL: 0.12 in Vert LL: 0.06 in Horz TL: 0 in	L/551 L/999	(3-4) (3-4) 3	L/240 L/360

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	3.5 in	1.50 in	569 lbs	-	-53 lbs	-292 lbs	-292 lbs	170 lbs
3	1	1.5 in	--	380 lbs	-	-102 lbs	-217 lbs	-217 lbs	-

03/17/2020

Material

TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 4
Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



Loads

- 1) This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL = 1.15.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) Minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web
		1.4 0.046 -449 lbs

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.00 has been applied for this truss analysis.
- 6) Listed wind uplift reactions based on MWFRS & C&C loading.

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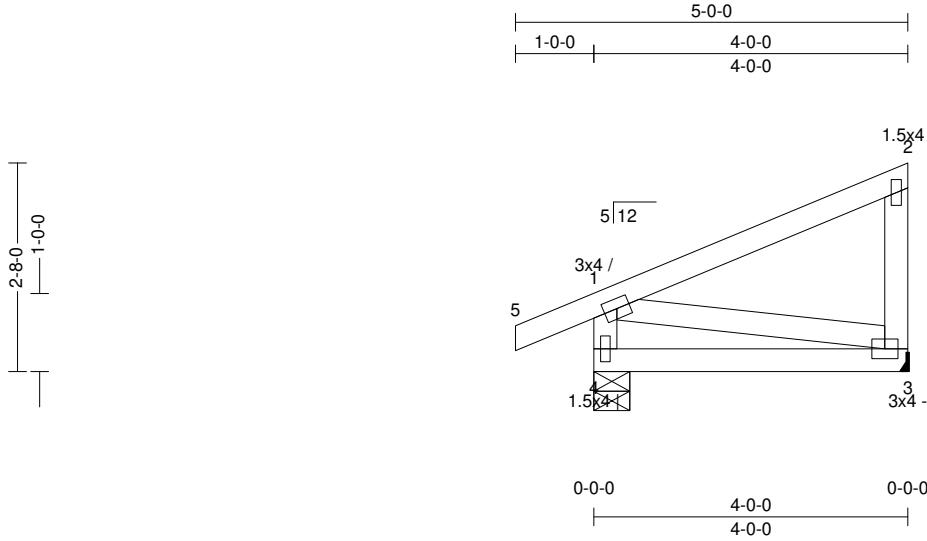
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Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:40

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SPAN 4-0-0	PITCH 5/12	QTY 4	OHL 1-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 17 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IRC 2015/	TC: 0.21 (1-2)	Vert TL: 0.02 in	L/999	(3-4)	L/240
TCDL: 15	TP1 1-2014	BC: 0.12 (3-4)	Vert LL: 0.01 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.08 (2-3)	Horz TL: 0 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

03/17/2020

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	5.5 in	1.50 in	366 lbs	-	-41 lbs	-240 lbs	-240 lbs	130 lbs
3	1	1.5 in	--	244 lbs	-	-73 lbs	-140 lbs	-140 lbs	-

Material

TC: SPF 1650/1.5 2 x 4
 BC: SPF 1650/1.5 2 x 4
 Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



Loads

- 1) This truss has been designed for the effects of balanced (30 psf) roof snow loads. in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/CtCs = 1, DOL= 1.15.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
- 5) Minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.00 has been applied for this truss analysis.
- 6) Listed wind uplift reactions based on MWFRS & C&C loading.

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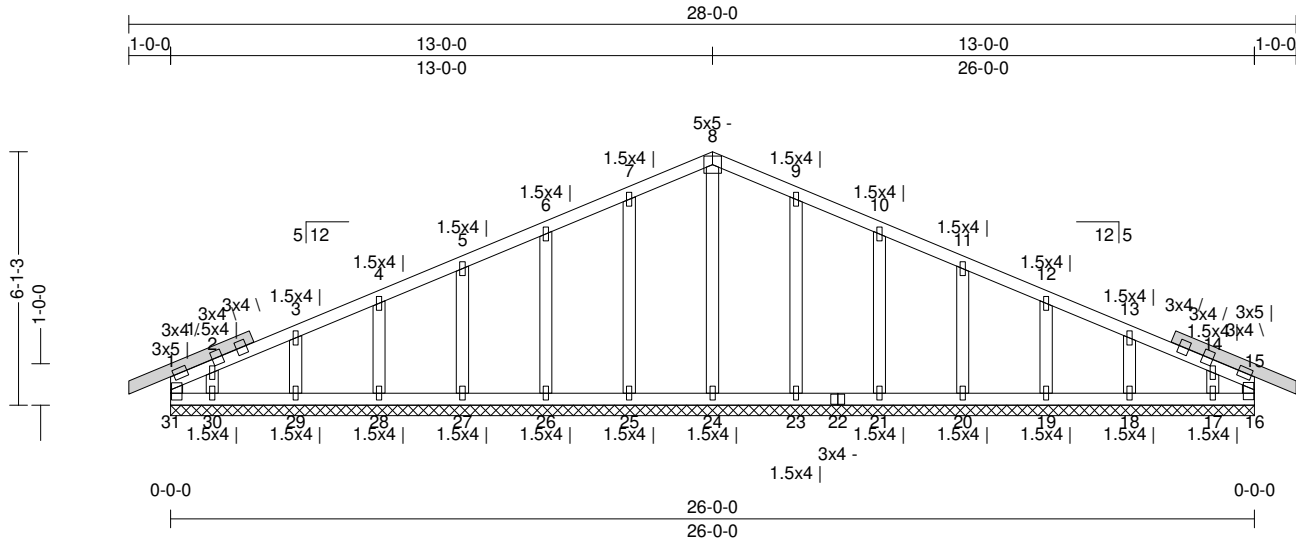
Truss: L01

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:44:54

Page: 1 of 1

SPAN 26-0-0	PITCH 5/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 111 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30 TCDL: 15 BCLL: 0 BCDL: 10	Bldg Code: IRC 2015/ TPI 1-2014 Rep Mbr: No Lumber D.O.L.: 115 %	TC: 0.21 (1-2) BC: 0.02 (17-18) Web: 0.10 (1-31)	Vert TL: 0 in UP Vert LL: 0 in Horz TL: 0 in	L/999 L/999	16 16	L/240 L/360

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		306 lbs	139 plf	-5 lbs	-65 lbs	-86 lbs	-86 lbs	-121 lbs

Material

TC: SPF 1650/1.5 2 x 4
 BC: SPF 1650/1.5 2 x 4
 Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/Cts = 1, DOL = 1.15.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	
BC	
Web	

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 1.5x4 20ga plates, U.N.O.
- Stitch top chords together with 3x4 20Ga plates at 24 in oc maximum, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 1.00 has been applied for this truss analysis.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 17, 30 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.



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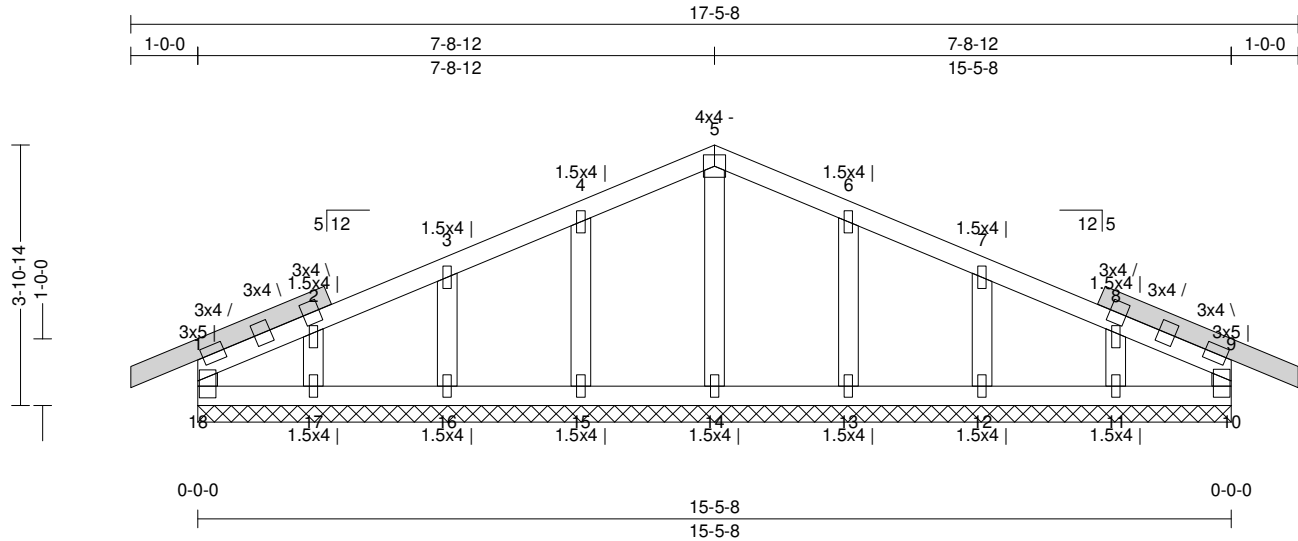
Truss: L02

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:44:57

Page: 1 of 1

SPAN 15-5-8	PITCH 5/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 61 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IRC 2015/	TC: 0.21 (1-2)	Vert TL: 0 in UP	L/999	10	L/240
TCDL: 15	TP1 1-2014	BC: 0.02 (15-16)	Vert LL: 0 in	L/999	10	L/360
BCLL: 0	Rep Mbr: No	Web: 0.10 (1-18)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		305 lbs	152 plf		-94 lbs	-175 lbs	-175 lbs	-114 lbs

Material

TC: SPF 1650/1.5 2 x 4
 BC: SPF 1650/1.5 2 x 4
 Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/CtCs = 1, DOL = 1.15.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Partial, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 1.5x4 20ga plates, U.N.O.
- Stitch top chords together with 3x4 20Ga plates at 24 in oc maximum, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 1.00 has been applied for this truss analysis.
- Listed wind uplift reactions based on MWFRS & C&C loading.



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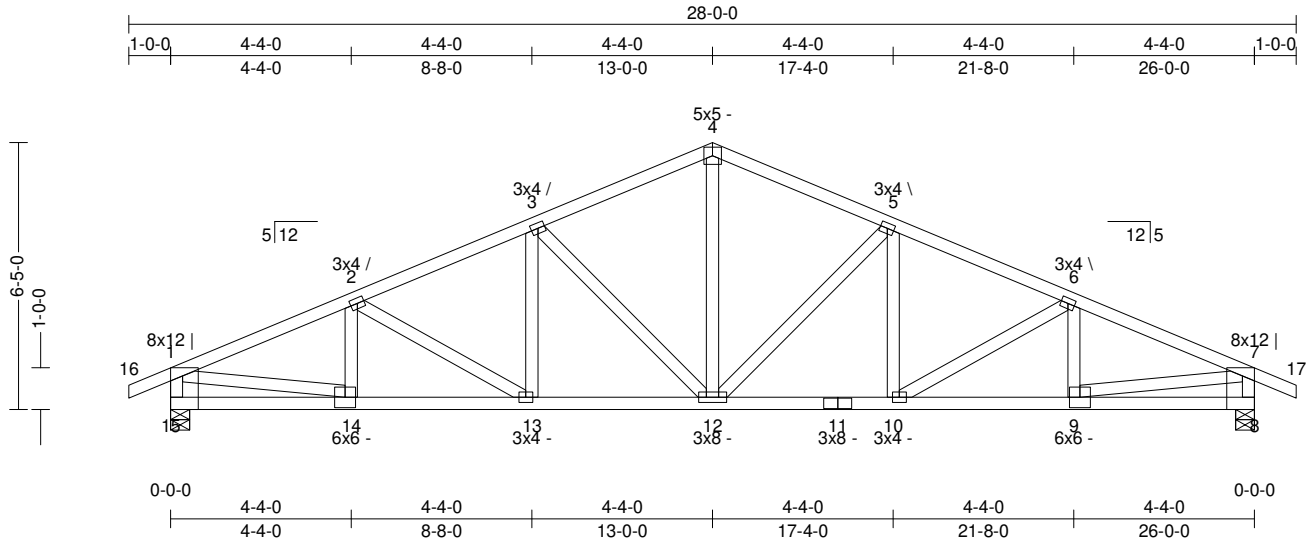
Truss: T01

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:44:59

Page: 1 of 1

SPAN 26-0-0	PITCH 5/12	QTY 11	OHL 1-0-0	OHR 1-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 120 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30 TCDL: 15 BCLL: 0 BCDL: 10	Bldg Code: IRC 2015/ TPI 1-2014 Rep Mbr: Yes Lumber D.O.L.: 115 %	TC: 0.32 (5-6) BC: 0.45 (10-12) Web: 0.40 (7-9)	Vert TL: 0.2 in Vert LL: 0.09 in Horz TL: 0.06 in	L/999 L/999 8	(12-13) 12 8	L/240 L/360

03/17/2020

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	5.5 in	2.84 in	1,813 lbs	-	-298 lbs	-462 lbs	-462 lbs	-33 lbs
8	1	5.5 in	2.84 in	1,813 lbs	-	-298 lbs	-462 lbs	-462 lbs	-

Material

TC: SPF 1650/1.5 2 x 4
 BC: SPF 1650/1.5 2 x 4
 Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 4-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



Loads

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/Cts = 1, DOL = 1.15.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.256	-2,712 lbs	3-4	0.304	-2,004 lbs	5-6	0.318	-2,562 lbs
	2-3	0.318	-2,562 lbs	4-5	0.304	-2,004 lbs	6-7	0.256	-2,712 lbs
BC	9-10	0.450	2,443 lbs	12-13	0.453	2,291 lbs			
	10-12	0.453	2,291 lbs	13-14	0.450	2,443 lbs			
Web	1-15	0.170	-1,734 lbs	5-12	0.358	-828 lbs			
	1-14	0.403	2,483 lbs	7-9	0.403	2,483 lbs			
	3-12	0.358	-828 lbs	7-8	0.170	-1,734 lbs			
	4-12	0.176	1,082 lbs						

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.00 has been applied for this truss analysis.
- Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild@Truss Software v5.6.355
 Eagle Metal Products

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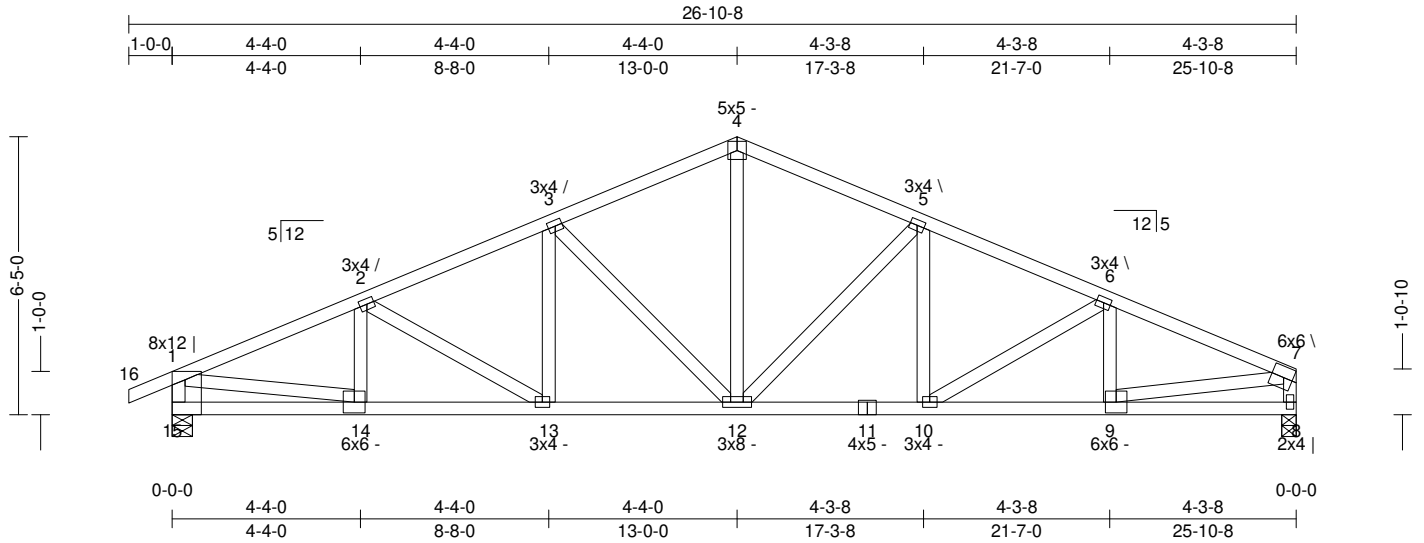
Truss: T02

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:02

Page: 1 of 1

SPAN 25-10-8	PITCH 5/12	QTY 10	OHL 1-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 118 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IRC 2015/	TC: 0.32 (2-3)	Vert TL: 0.2 in	L/999	(12-13)	L/240
TCDL: 15	TP1 1-2014	BC: 0.45 (12-13)	Vert LL: 0.09 in	L/999	(12-13)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.40 (1-14)	Horz TL: 0.06 in		8	
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	5.5 in	2.84 in	1,808 lbs		-297 lbs	-461 lbs	-461 lbs	55 lbs
8	1	4 in	2.69 in	1,713 lbs		-264 lbs	-376 lbs	-376 lbs	

Material

TC: SPF 1650/1.5 2 x 4
 BC: SPF 1650/1.5 2 x 4
 Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 4-2-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/Cs = 1, DOL = 1.15.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Minimum storage attic loading has been applied in accordance with IRC 301.5

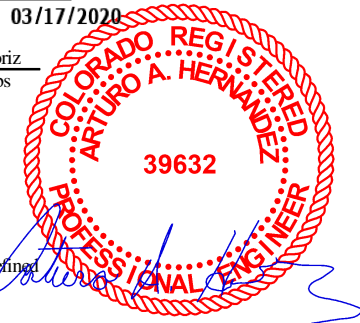
Member Forces

Table indicates: Member ID, max CSI max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.255	-2,703 lbs	3-4	0.304	-1,990 lbs	5-6	0.307	-2,531 lbs
	2-3	0.318	-2,550 lbs	4-5	0.294	-1,989 lbs	6-7	0.284	-2,660 lbs
BC	9-10	0.441	2,402 lbs (-459 lbs)	12-13	0.450	2,280 lbs (-374 lbs)			
	10-12	0.447	2,261 lbs (-371 lbs)	13-14	0.449	2,434 lbs (-457 lbs)			
Web	1-15	0.169	-1,728 lbs	5-12	0.348	-809 lbs			
	1-14	0.402	2,473 lbs (-442 lbs)	6-9	0.037	-314 lbs			
	3-12	0.358	-829 lbs	7-9	0.397	2,447 lbs (-490 lbs)			
	4-12	0.174	1,073 lbs (-257 lbs)	7-8	0.160	-1,634 lbs			

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.00 has been applied for this truss analysis.
- Listed wind uplift reactions based on MWFRS & C&C loading.



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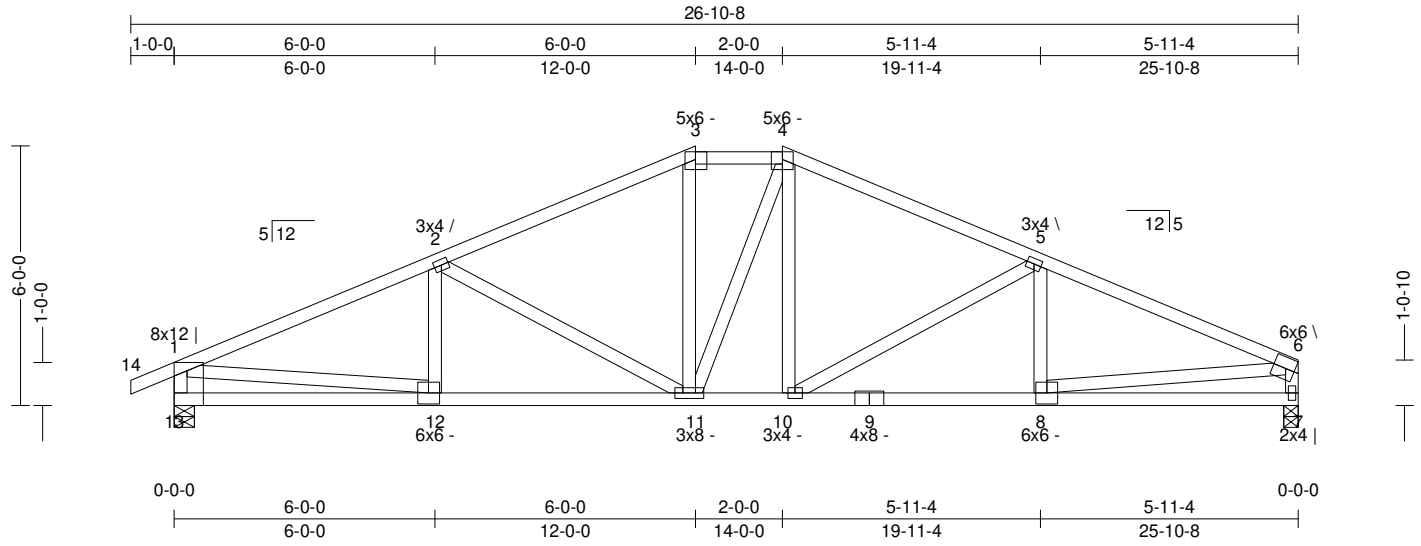
Truss: T03

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:04

Page: 1 of 1

SPAN 25-10-8	PITCH 5/12	QTY 1	OHL 1-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 117 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30 TCDL: 15 BCLL: 0 BCDL: 10	Bldg Code: IRC 2015/ TPI 1-2014 Rep Mbr: No Lumber D.O.L.: 115 %	TC: 0.62 (5-6) BC: 0.66 (11-12) Web: 0.41 (1-12)	Vert TL: 0.23 in Vert LL: 0.09 in Horz TL: 0.05 in	L/999 L/999	(8-9) (9-10) 7	L/240 L/360

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
13	1	5.5 in	2.83 in	1,802 lbs	-	-297 lbs	-450 lbs	-450 lbs	55 lbs
7	1	4 in	2.70 in	1,719 lbs	-	-264 lbs	-365 lbs	-365 lbs	-

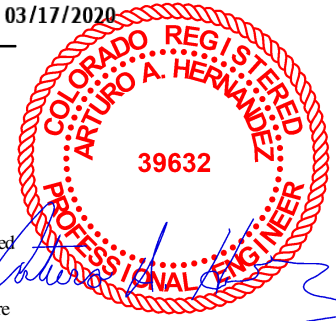
03/17/2020

Material

TC: SPF 1650/1.5 2 x 4
 BC: SPF 1650/1.5 2 x 4
 Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 3-6-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



Loads

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE 7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/Cts = 1, DOL = 1.15.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE 7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

Member	ID	Force	Direction	ID	Force	Direction	ID	Force	Direction
TC	1-2	0.618	-2,824 lbs	3-4	0.427	-1,927 lbs	5-6	0.621	-2,812 lbs
	2-3	0.598	-2,183 lbs	4-5	0.602	-2,216 lbs			
BC	8-10	0.659	2,524 lbs	11-12	0.662	2,530 lbs			
	10-11	0.466	1,932 lbs						
Web	1-13	0.166	-1,699 lbs	3-11	0.093	575 lbs	5-10	0.353	-693 lbs
	1-12	0.414	2,552 lbs	4-11	0.114	313 lbs	6-8	0.414	2,549 lbs
	2-11	0.397	-768 lbs	4-10	0.079	484 lbs	6-7	0.159	-1,618 lbs

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.00 has been applied for this truss analysis.
- Listed wind uplift reactions based on MWFRS & C&C loading.

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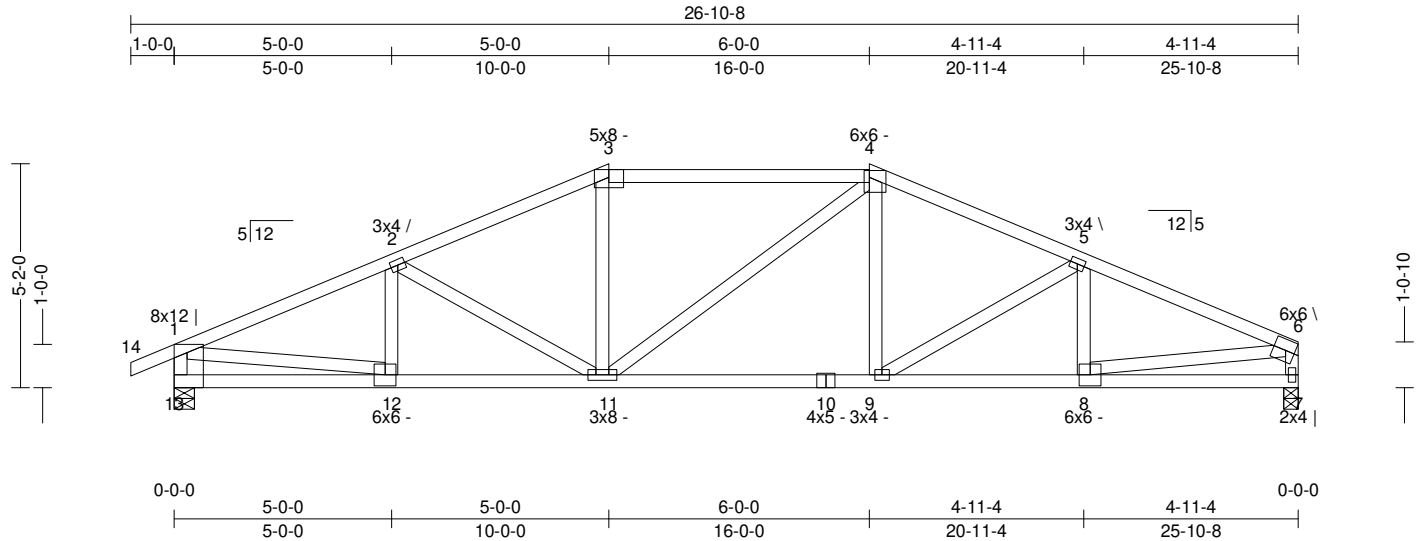
Truss: T04

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:07

Page: 1 of 1

SPAN 25-10-8	PITCH 5/12	QTY 1	OHL 1-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 111 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30 TCDL: 15 BCLL: 0 BCDL: 10	Bldg Code: IRC 2015/ TPI 1-2014 Rep Mbr: No Lumber D.O.L.: 115 %	TC: 0.62 (3-4) BC: 0.63 (11-12) Web: 0.41 (1-12)	Vert TL: 0.25 in Vert LL: 0.09 in Horz TL: 0.06 in	L/999 L/999	(10-11) (10-11) 7	L/240 L/360

03/17/2020

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
13	1	5.5 in	2.83 in	1,802 lbs	.	-297 lbs	-428 lbs	-428 lbs	55 lbs
7	1	4 in	2.70 in	1,719 lbs	.	-264 lbs	-342 lbs	-342 lbs	.

Material

TC: SPF 1650/1.5 2 x 4
 BC: SPF 1650/1.5 2 x 4
 Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 3-7-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



Loads

- 1) This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/Cts = 1, DOL = 1.15.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) Minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.450	-2,751 lbs	3-4	0.621	-2,177 lbs	5-6	0.499	-2,724 lbs
	2-3	0.536	-2,436 lbs	4-5	0.596	-2,478 lbs			
BC	8-9	0.616	2,451 lbs (-405 lbs)	11-12	0.625	2,471 lbs (-403 lbs)			
	9-11	0.590	2,225 lbs (-272 lbs)						
Web	1-13	0.168	-1,710 lbs	3-11	0.072	443 lbs (-6 lbs)	6-8	0.404	2,485 lbs (-433 lbs)
	1-12	0.406	2,500 lbs (-385 lbs)	4-11	0.205	326 lbs (-286 lbs)	6-7	0.160	-1,629 lbs
	2-11	0.131	-361 lbs	4-9	0.060	371 lbs (-11 lbs)			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.00 has been applied for this truss analysis.
- 6) Listed wind uplift reactions based on MWFRS & C&C loading.

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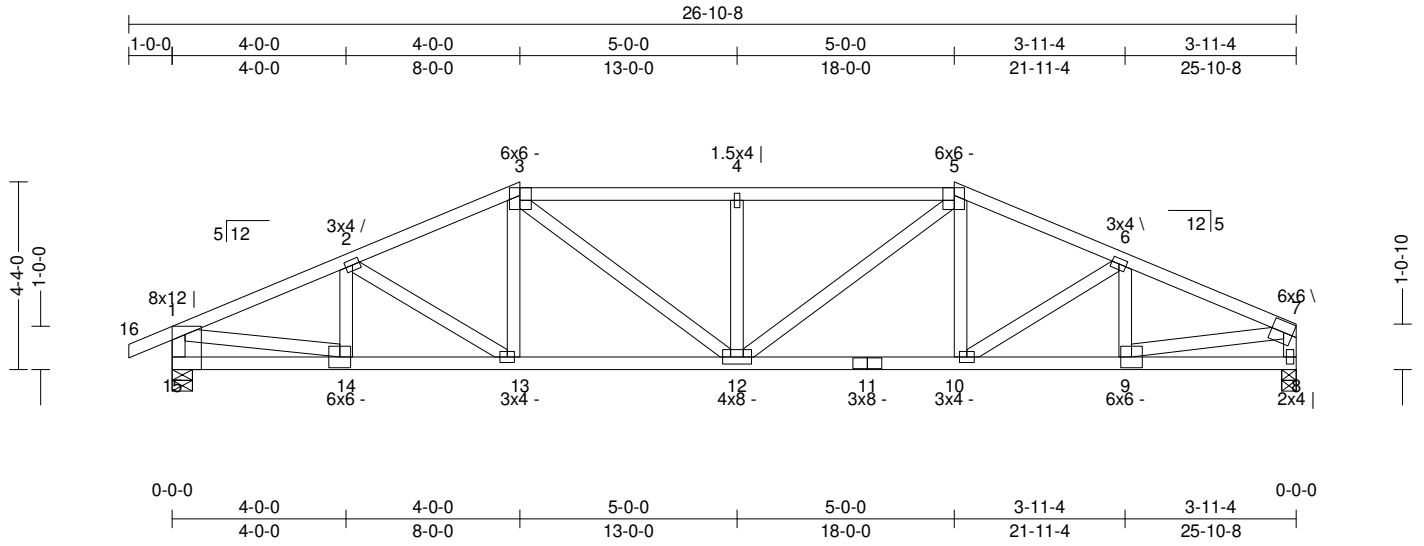
Truss: T05

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:09

Page: 1 of 1

SPAN 25-10-8	PITCH 5/12	QTY 1	OHL 1-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 113 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30 TCDL: 15 BCLL: 0 BCDL: 10	Bldg Code: IRC 2015/ TP1 1-2014 Rep Mbr: No Lumber D.O.L.: 115 %	TC: 0.52 (4-5) BC: 0.55 (12-13) Web: 0.39 (1-14)	Vert TL: 0.24 in Vert LL: 0.1 in Horz TL: 0.06 in	L/999 L/999 8	(12-13) 12 8	L/240 L/360

Reaction

03/17/2020

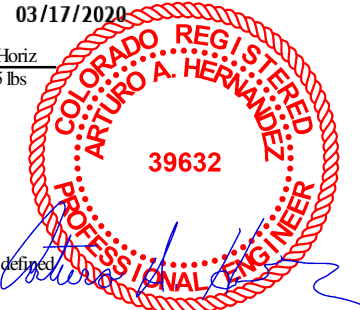
JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	5.5 in	2.83 in	1,802 lbs	-	-297 lbs	-405 lbs	-405 lbs	55 lbs
8	1	4 in	2.70 in	1,719 lbs	-	-264 lbs	-320 lbs	-320 lbs	-

Material

TC: SPF 1650/1.5 2 x 4
 BC: SPF 1650/1.5 2 x 4
 Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 3-6-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



Loads

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/CtCs = 1, DOL = 1.15.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.284	-2,629 lbs	3-4	0.502	-2,790 lbs	5-6	0.442	-2,632 lbs
	2-3	0.462	-2,617 lbs	4-5	0.522	-2,790 lbs	6-7	0.337	-2,598 lbs
BC	9-10	0.517	2,344 lbs	12-13	0.550	2,373 lbs			
	10-12	0.550	2,392 lbs	13-14	0.520	2,366 lbs			
Web	1-15	0.169	-1,724 lbs	4-12	0.105	-536 lbs	7-8	0.161	-1,643 lbs
	1-14	0.391	2,411 lbs	5-12	0.131	806 lbs			
	2-14	0.037	-325 lbs	6-9	0.041	-358 lbs			
	3-12	0.137	844 lbs	7-9	0.389	2,397 lbs			

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- Provide adequate drainage to prevent ponding.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.00 has been applied for this truss analysis.
- Listed wind uplift reactions based on MWFRS & C&C loading.

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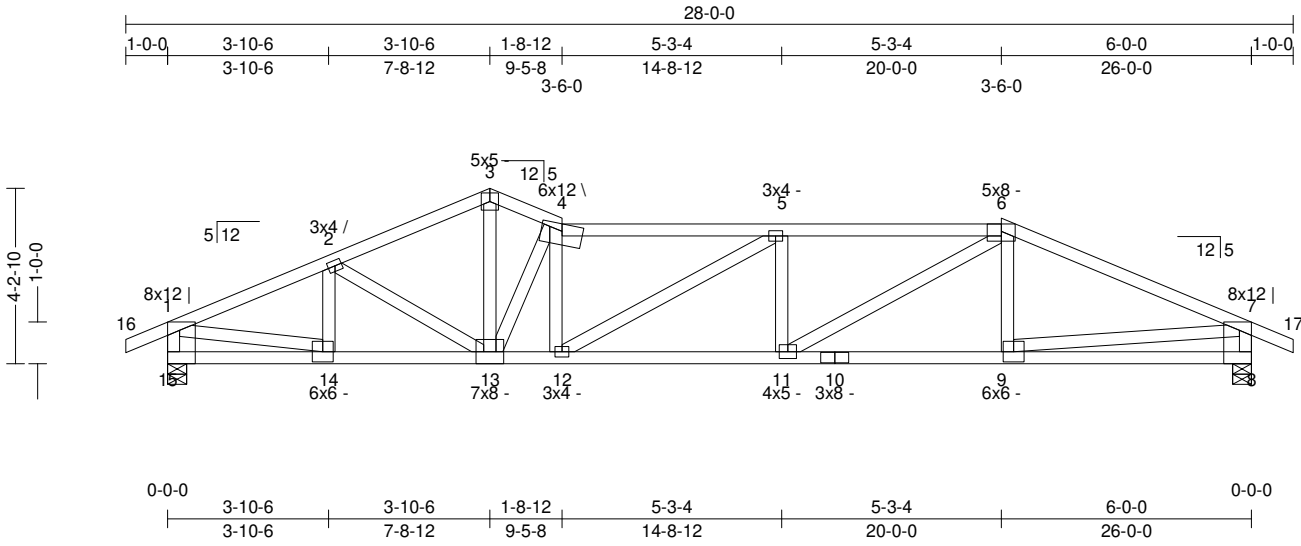
Truss: T06

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:12

Page: 1 of 1

SPAN 26-0-0	PITCH 5/12	QTY 1	OHL 1-0-0	OHR 1-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 118 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30 TCDL: 15 BCLL: 0 BCDL: 10	Bldg Code: IRC 2015/ TPI 1-2014 Rep Mbr: No Lumber D.O.L.: 115 %	TC: 0.54 (5-6) BC: 0.79 (11-12) Web: 0.43 (7-9)	Vert TL: 0.38 in Vert LL: 0.17 in Horz TL: 0.07 in	L / 792 L / 999	(11-12) (11-12) 8	L / 240 L / 360

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	5.5 in	2.79 in	1,780 lbs	-	-298 lbs	-410 lbs	-410 lbs	-36 lbs
8	1	5.5 in	2.90 in	1,847 lbs	-	-298 lbs	-395 lbs	-395 lbs	-

Material

TC: SPF 1650/1.5 2 x 4
 BC: SPF 1650/1.5 2 x 4
 Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 3-1-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/CtCs = 1, DOL = 1.15.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.282	-2,583 lbs	3-4	0.315	-2,637 lbs	5-6	0.541	-3,755 lbs
	2-3	0.280	-2,601 lbs	4-5	0.524	-3,487 lbs	6-7	0.499	-2,901 lbs
BC	9-11	0.654	2,597 lbs (-323 lbs)	12-13	0.657	3,528 lbs (-443 lbs)			
	11-12	0.794	3,755 lbs (-480 lbs)	13-14	0.466	2,327 lbs (-326 lbs)			
Web	1-15	0.167	-1,707 lbs	4-13	0.357	-2,103 lbs	7-9	0.426	2,624 lbs (-332 lbs)
	1-14	0.386	2,375 lbs (-352 lbs)	5-12	0.159	-356 lbs	7-8	0.171	-1,747 lbs
	2-14	0.037	-330 lbs	5-11	0.062	-445 lbs			
	3-13	0.291	1,790 lbs (-287 lbs)	6-11	0.215	1,323 lbs (-179 lbs)			

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- Provide adequate drainage to prevent ponding.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.00 has been applied for this truss analysis.
- Listed wind uplift reactions based on MWFRS & C&C loading.



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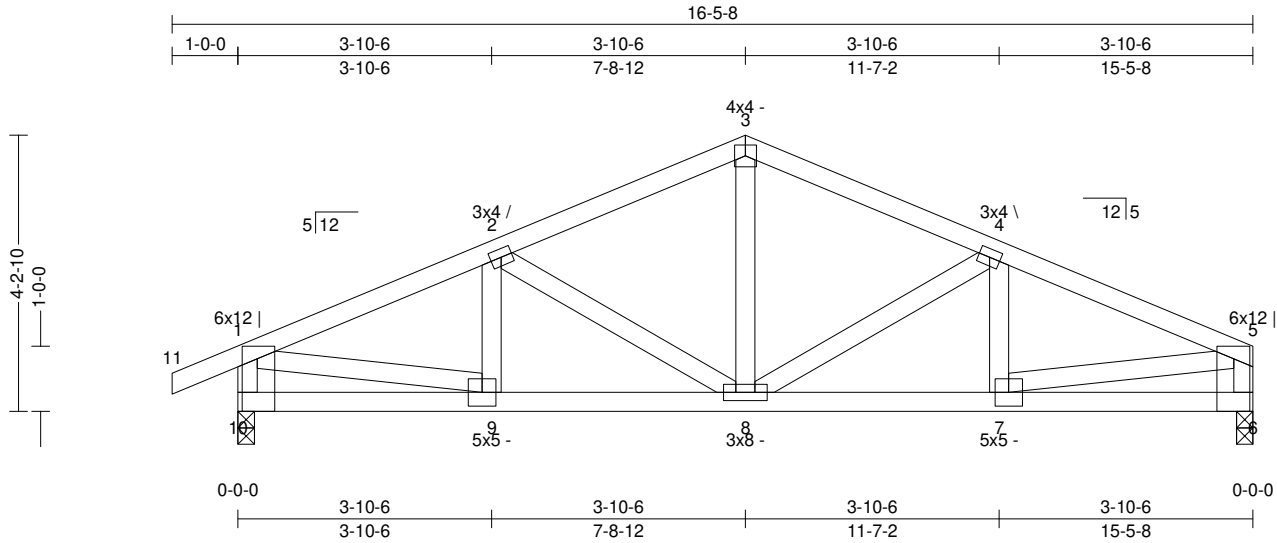
Truss: T07

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:14

Page: 1 of 1

SPAN 15-5-8	PITCH 5/12	QTY 3	OHL 1-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 67 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30 TCDL: 15 BCLL: 0 BCDL: 10	Bldg Code: IRC 2015/ TP1 1-2014 Rep Mbr: Yes Lumber D.O.L.: 115 %	TC: 0.26 (4-5) BC: 0.29 (7-8) Web: 0.23 (5-7)	Vert TL: 0.06 in Vert LL: 0.03 in UP Horz TL: 0.01 in	L/999 L/999	(7-8) (7-8) 6	L/240 L/360

03/17/2020

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
10	1	3 in	1.90 in	1,211 lbs	-	-312 lbs	-627 lbs	-627 lbs	53 lbs
6	1	3 in	1.75 in	1,115 lbs	-	-264 lbs	-521 lbs	-521 lbs	-

Material

TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 4
Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 5-7-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



Loads

- 1) This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/CtCs = 1, DOL = 1.15.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Partial, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered, DOL = 1.60
- 4) Minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

Member	ID	CSI	Max Axial Force	Max Compr. Force
TC	1-2	0.239	-1,557 lbs	-
	2-3	0.236	-1,155 lbs	-
	3-4	0.254	-1,160 lbs	-
BC	4-5	0.258	-1,572 lbs	-
	7-8	0.295	1,392 lbs	(-559 lbs)
	8-9	0.291	1,371 lbs	(-542 lbs)
Web	1-10	0.112	-1,140 lbs	-
	1-9	0.227	1,399 lbs	(-534 lbs)
	2-8	0.111	-462 lbs	-
	5-6	0.102	-1,044 lbs	(-220 lbs)
	3-8	0.076	471 lbs	-
	4-8	0.116	-484 lbs	-
	5-7	0.231	1,421 lbs	(-602 lbs)

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 1.00 has been applied for this truss analysis.
- 5) Listed wind uplift reactions based on MWFRS & C&C loading.

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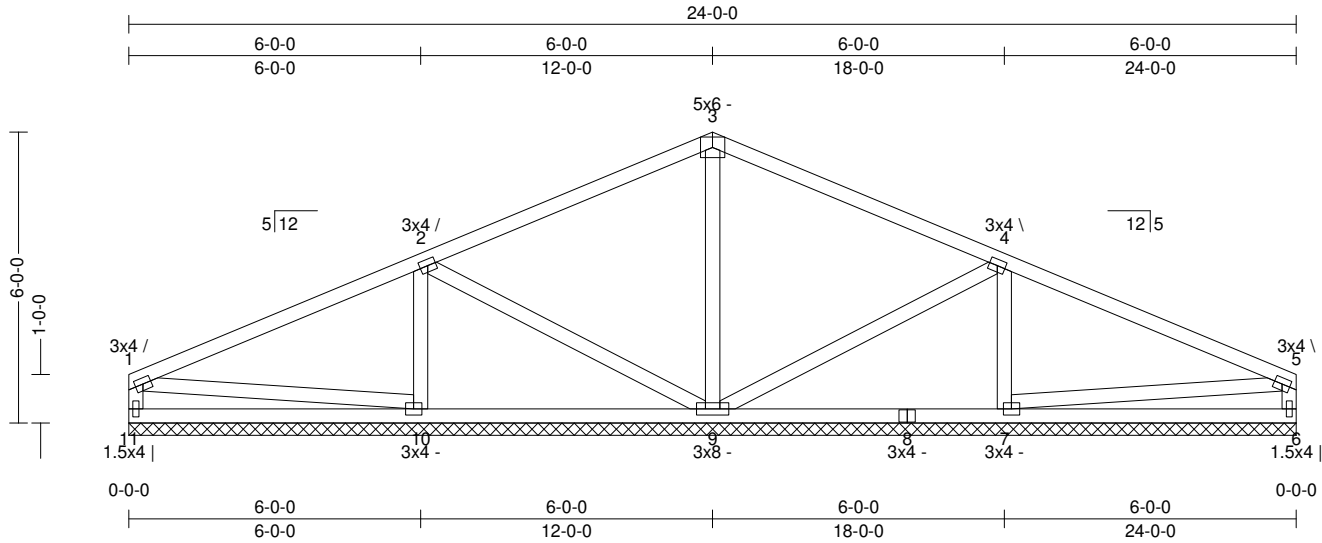
Truss: T08

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:16

Page: 1 of 1

SPAN 24-0-0	PITCH 5/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 95 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IRC 2015/	TC: 0.67 (4-5)	Vert TL: 0.08 in	L/999	(6-7)	L/240
TCDL: 15	TPF 1-2014	BC: 0.31 (10-11)	Vert LL: 0.04 in	L/999	(6-7)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.19 (3-9)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

03/17/2020

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	288 in	N/A	367 lbs	-	-46 lbs	-69 lbs	-69 lbs	-21 lbs
1	288 in	N/A	922 lbs	-	-130 lbs	-183 lbs	-183 lbs	-108 lbs
1	288 in	N/A	41 lbs	-	-	-	-	-
1	288 in	N/A	781 lbs	-	-160 lbs	-218 lbs	-218 lbs	-62 lbs
1	288 in	N/A	957 lbs	-	-119 lbs	-172 lbs	-172 lbs	108 lbs
1	288 in	N/A	363 lbs	-	-47 lbs	-71 lbs	-71 lbs	21 lbs



Material

TC: SPF 1650/1.5 2 x 4
 BC: SPF 1650/1.5 2 x 4
 Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL = 1.15.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web	2-10	0.102	-699 lbs
			3-9	0.193	-500 lbs
			4-7	0.102	-699 lbs

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.00 has been applied for this truss analysis.
- Listed wind uplift reactions based on MWFRS & C&C loading.

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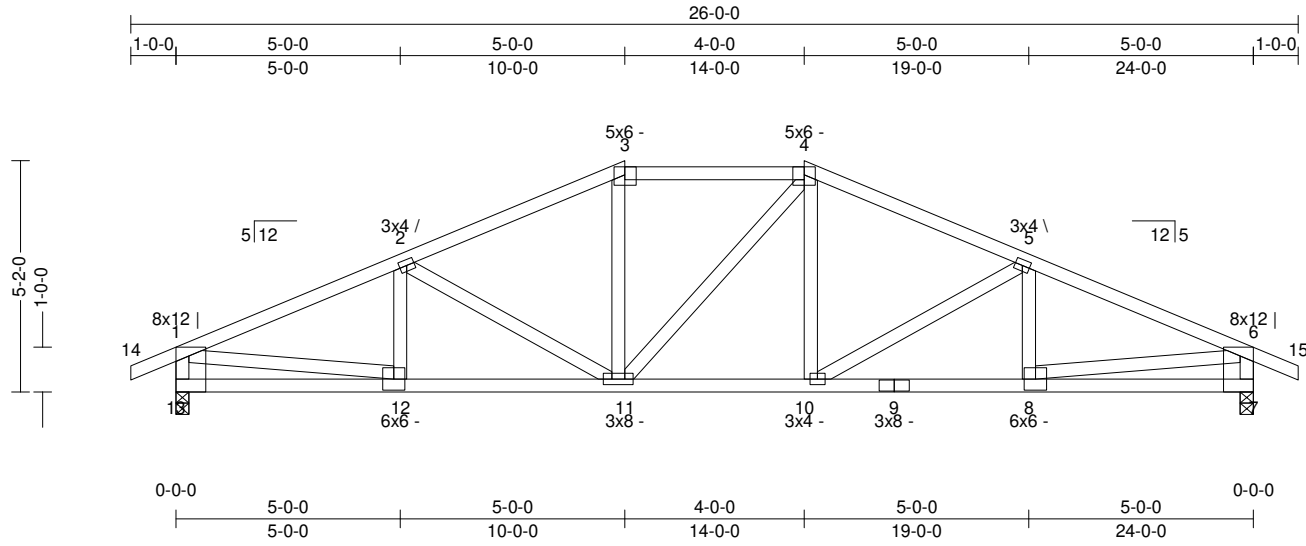
Truss: T09

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:19

Page: 1 of 1

SPAN 24-0-0	PITCH 5/12	QTY 1	OHL 1-0-0	OHR 1-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 108 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30 TCDL: 15 BCLL: 0 BCDL: 10	Bldg Code: IRC 2015/ TP1 1-2014 Rep Mbr: No Lumber D.O.L.: 115 %	TC: 0.39 (5-6) BC: 0.52 (8-10) Web: 0.38 (6-8)	Vert TL: 0.17 in Vert LL: 0.07 in Horz TL: 0.05 in	L/999 L/999	9 (10-11) 7	L/240 L/360

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
13	1	3.5 in	2.66 in	1,697 lbs	-	-277 lbs	-410 lbs	-410 lbs	-33 lbs
7	1	3.5 in	2.66 in	1,697 lbs	-	-277 lbs	-410 lbs	-410 lbs	-

Material

TC: SPF 1650/1.5 2 x 4
 BC: SPF 1650/1.5 2 x 4
 Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 4-0-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/Cts = 1, DOL = 1.15.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

Member	ID	CSI	Max Axial Force	Max Compr. Force
TC	1-2	0.389	-2,565 lbs	
	2-3	0.382	-2,173 lbs	
	3-4	0.381	-1,908 lbs	
BC	8-10	0.519	2,302 lbs	(-331 lbs)
	10-11	0.445	1,913 lbs	(-191 lbs)
	11-12	0.518	2,301 lbs	(-331 lbs)
Web	1-13	0.158	-1,608 lbs	
	1-12	0.378	2,329 lbs	(-356 lbs)
	2-11	0.168	-461 lbs	
	3-11	0.076	-468 lbs	(-29 lbs)
	4-10	0.067	-410 lbs	(-29 lbs)

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.00 has been applied for this truss analysis.
- Listed wind uplift reactions based on MWFRS & C&C loading.



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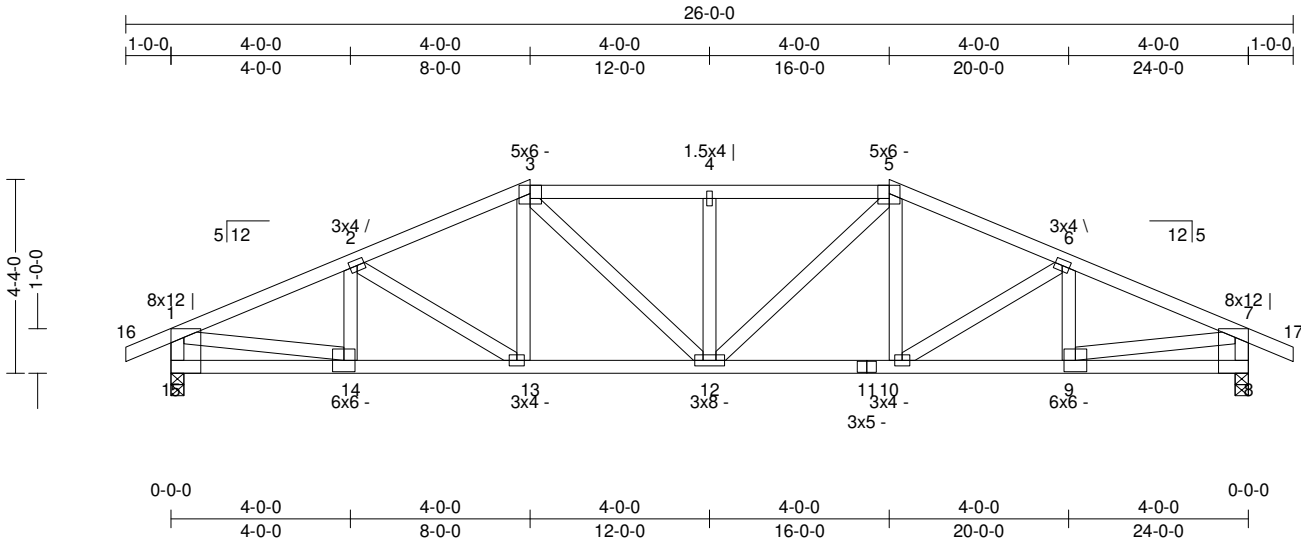
Truss: T10

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:22

Page: 1 of 1

SPAN 24-0-0	PITCH 5/12	QTY 1	OHL 1-0-0	OHR 1-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 109 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30 TCDL: 15 BCLL: 0 BCDL: 10	Bldg Code: IRC 2015/ TPI 1-2014 Rep Mbr: No Lumber D.O.L.: 115 %	TC: 0.35 (3-4) BC: 0.46 (9-10) Web: 0.37 (7-9)	Vert TL: 0.18 in Vert LL: 0.08 in Horz TL: 0.05 in	L/999 L/999 8	(11-12) 12 8	L/240 L/360

03/17/2020

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	3.5 in	2.66 in	1,697 lbs	-	-277 lbs	-388 lbs	-388 lbs	-33 lbs
8	1	3.5 in	2.66 in	1,697 lbs	-	-277 lbs	-388 lbs	-388 lbs	-

Material

TC: SPF 1650/1.5 2 x 4
 BC: SPF 1650/1.5 2 x 4
 Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 4-2-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



Loads

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/CtCs = 1, DOL = 1.15.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.252	-2,454 lbs	3-4	0.352	-2,367 lbs	5-6	0.349	-2,375 lbs
	2-3	0.349	-2,375 lbs	4-5	0.352	-2,367 lbs	6-7	0.252	-2,454 lbs
BC	9-10	0.459	2,207 lbs	12-13	0.448	2,139 lbs			
	10-12	0.448	2,139 lbs	13-14	0.459	2,207 lbs			
Web	1-15	0.159	-1,621 lbs	5-12	0.101	622 lbs			
	1-14	0.365	2,249 lbs	7-9	0.365	2,249 lbs			
	3-12	0.101	622 lbs	7-8	0.159	-1,621 lbs			
	4-12	0.076	-391 lbs						

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Provide adequate drainage to prevent ponding.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.00 has been applied for this truss analysis.
- Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild@Truss Software v5.6.355
 Eagle Metal Products

Habitat for Humanity of Colorado

P.O. Box 100
 Onley Springs, CO 81062
 (719) 267-5323

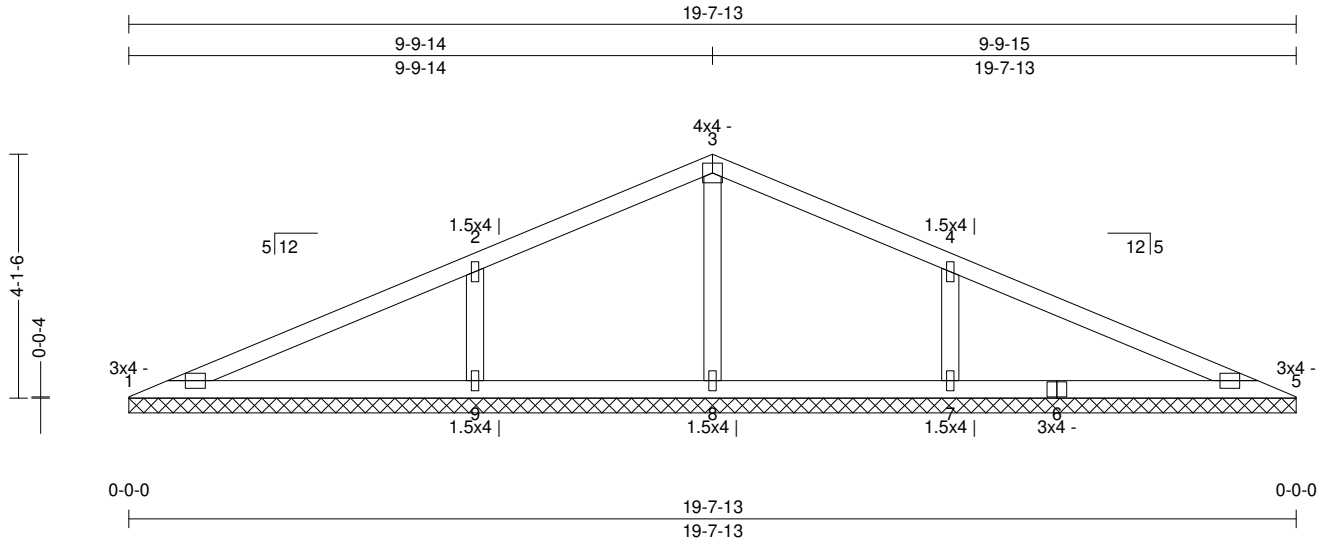
Truss: V01

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:24

Page: 1 of 1

SPAN 19-7-13	PITCH 5/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 52 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IRC 2015/	TC: 0.35 (1-2)	Vert TL: 0.01 in	L/999	(9-1)	L/240
TCDL: 15	TP1 1-2014	BC: 0.16 (9-1)	Vert LL: 0 in	L/999	(9-1)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.06 (2-9)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

03/17/2020

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		678 lbs	161 plf	-155 lbs	-159 lbs	-210 lbs	-210 lbs	329 lbs

Material

TC: SPF 1650/1.5 2 x 4
 BC: SPF 1650/1.5 2 x 4
 Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL = 1.15.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web
		29 0.064 -583 lbs 4-7 0.064 -582 lbs

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 48" OC, U.N.O.
- Attach gable webs with 1.5x4 20ga plates, U.N.O.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 1.00 has been applied for this truss analysis.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 1, 5, 1, 5 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.



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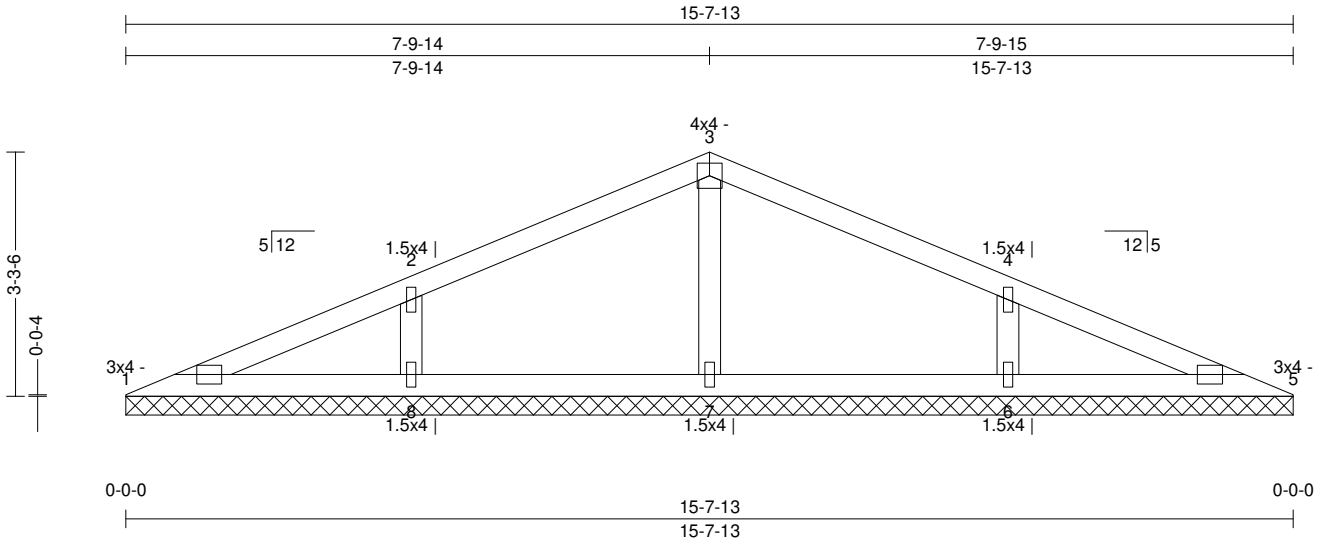
Truss: V02

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:26

Page: 1 of 1

SPAN 15-7-13	PITCH 5/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 40 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IRC 2015/	TC: 0.25 (4-5)	Vert TL: 0 in	L/999	(6-7)	L/240
TCDL: 15	TP1 1-2014	BC: 0.08 (6-7)	Vert LL: 0 in	L/999	5	L/360
BCLL: 0	Rep Mbr: No	Web: 0.05 (4-6)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

03/17/2020

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	552 lbs	140 plf	-15 lbs	-104 lbs	-242 lbs	-242 lbs	131 lbs

Material

TC: SPF 1650/1.5 2 x 4
BC: SPF 1650/1.5 2 x 4
Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL = 1.15.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web	2-8	0.048	479 lbs	3-7	0.044	-328 lbs	4-6	0.048	-479 lbs
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Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 48" OC, U.N.O.
- Attach gable webs with 1.5x4 20ga plates, U.N.O.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 1.00 has been applied for this truss analysis.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 1, 5, 1, 5 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.



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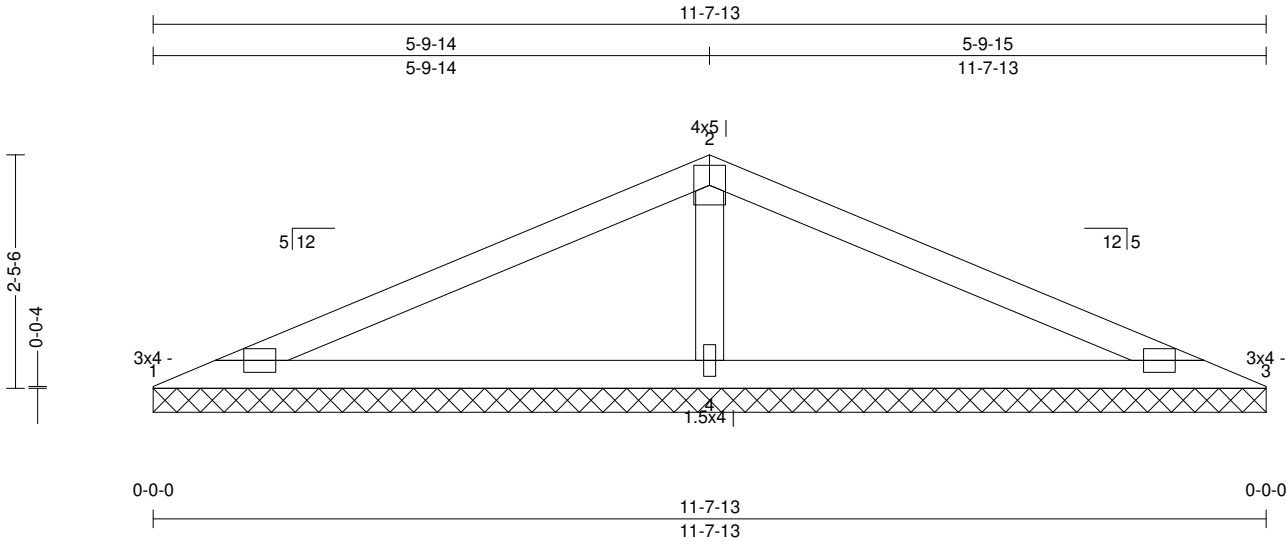
Truss: V03

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:28

Page: 1 of 1

SPAN 11-7-13	PITCH 5/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 27 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IRC 2015/	TC: 0.46 (2-3)	Vert TL: 0.01 in	L/999	(3-4)	L/240
TCDL: 15	TPI 1-2014	BC: 0.19 (3-4)	Vert LL: 0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.04 (2-4)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		830 lbs	204 plf	-192 lbs	-146 lbs	-379 lbs	-379 lbs	438 lbs

Material

TC: SPF 1650/1.5 2 x 4
 BC: SPF 1650/1.5 2 x 4
 Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (30 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), CeCtCs = 1, DOL = 1.15.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

Member	Force	Force	Force	Force
TC	1-2	0.458	343 lbs	(-270 lbs)
BC	2-3	0.458	343 lbs	(-270 lbs)
Web	2-4	0.036	-327 lbs	

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 48" OC, U.N.O.
- Attach gable webs with 3x4 20ga plates, U.N.O.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 1.00 has been applied for this truss analysis.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 1, 3, 1, 3 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.



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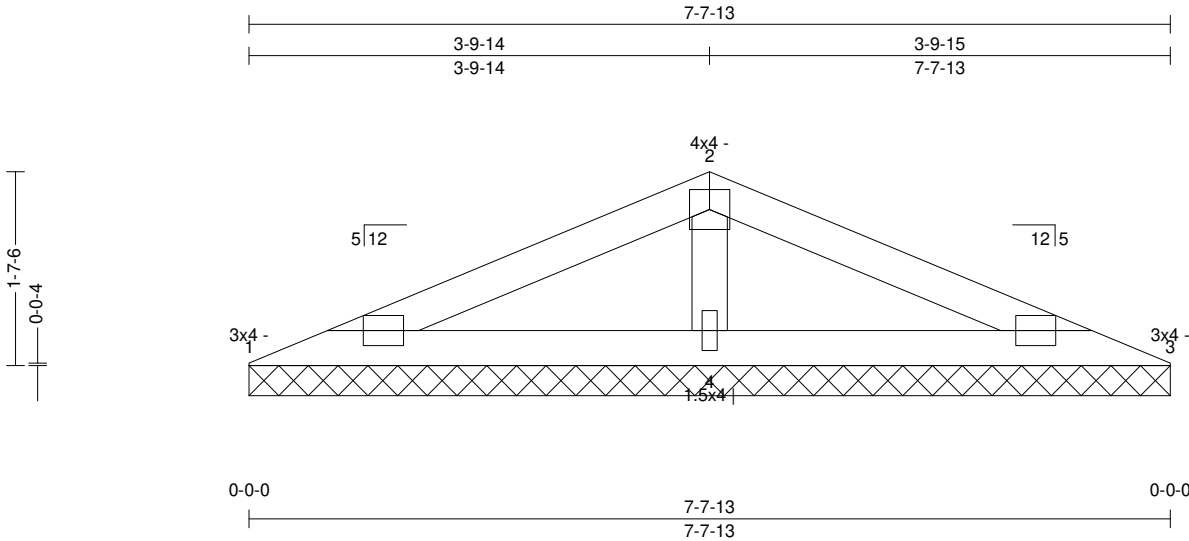
Truss: V04

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:30

Page: 1 of 1

SPAN 7-7-13	PITCH 5/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 18 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IRC 2015/	TC: 0.10 (1-2)	Vert TL: 0 in	L/999	(3-4)	L/240
TCDL: 15	TP1 1-2014	BC: 0.04 (3-4)	Vert LL: 0 in	L/999	3	L/360
BCLL: 0	Rep Mbr: No	Web: 0.02 (2-4)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	290 lbs	124 plf	-28 lbs	-72 lbs	-210 lbs	-210 lbs	152 lbs

Material

TC: SPF 1650/1.5 2 x 4
 BC: SPF 1650/1.5 2 x 4
 Web: SPF 1650/1.5 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (30 psf) roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/CtCs = 1, DOL = 1.15.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 48" OC, U.N.O.
- Attach gable webs with 3x4 20ga plates, U.N.O.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 1.00 has been applied for this truss analysis.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 1, 3, 1, 3 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.



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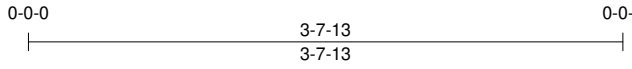
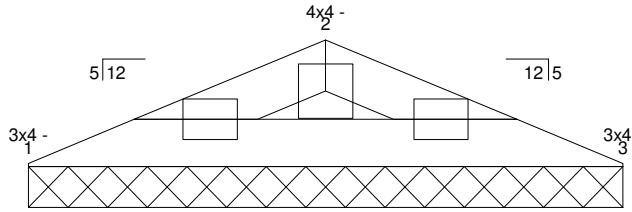
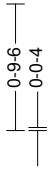
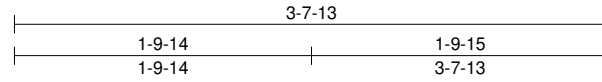
Truss: V05

Job: PP - 3410_0 - C Risley4Bdr1S

Date: 03/17/20 07:45:32

Page: 1 of 1

SPAN 3-7-13	PITCH 5/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 7lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IRC 2015/	TC: 0.02 (2-3)	Vert TL: 0 in	L/999	3	L/240
TCDL: 15	TP1 1-2014	BC: 0.01 (3-1)	Vert LL: 0 in	L/999	3	L/360
BCLL: 0	Rep Mbr: No	Web: 0.00 (1)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

03/17/2020

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		140 lbs	110 plf		-32 lbs	-104 lbs	-104 lbs	-89 lbs

Material

TC: SPF 1650/1.5 2 x 4
 BC: SPF 1650/1.5 2 x 4
 Web:

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (30 psf) roof snow loads, in accordance with ASCE7 - 10 with the following user defined input: 30 psf Roof (GSL = 30 psf), Ce/CtCs = 1, DOL = 1.15.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 130 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 48" OC, U.N.O.
- Attach gable webs with 3x4 20ga plates, U.N.O.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 1.00 has been applied for this truss analysis.
- Listed wind uplift reactions based on MWFRS & C&C loading.



ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild@Truss Software v5.6.355
 Eagle Metal Products

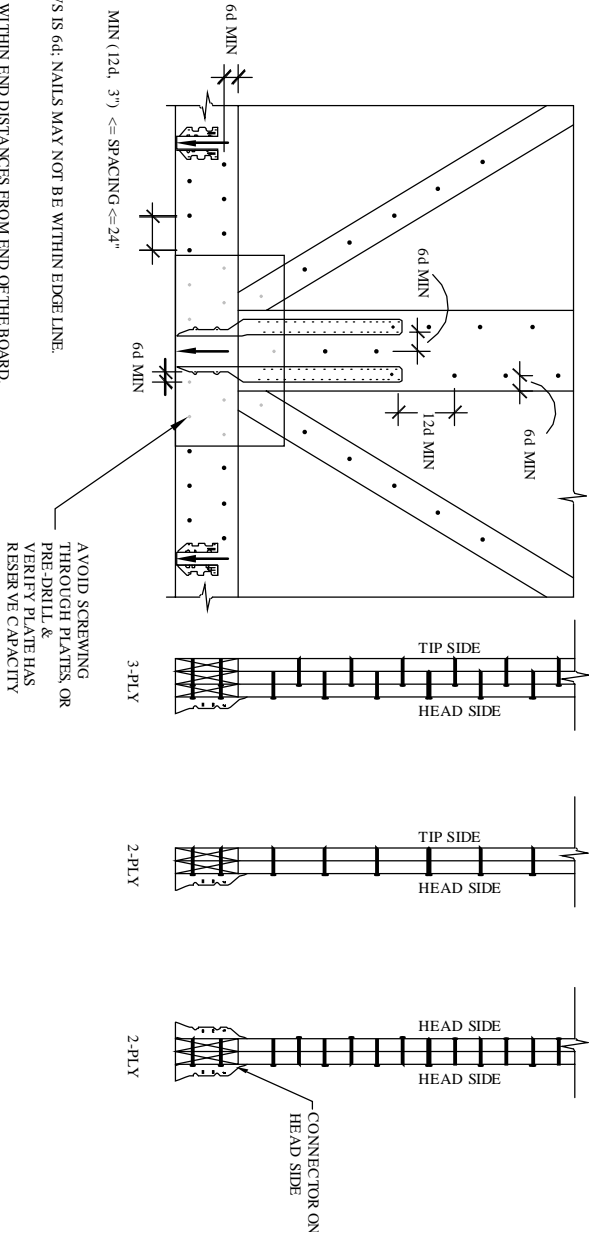
PANEL LOADS	8d, 2.5" BOX				0.120 GUN (3" MIN)				10, 12d BOX or 8d COMMON (3" MIN)				16d BOX				12d COMMON OR 2d BOX				16d COMMON			
	SP 2-PLY	SP 3-PLY	SPE 2-PLY	SPE 3-PLY	SP 2-PLY	SP 3-PLY	SPE 2-PLY	SPE 3-PLY	SP 2-PLY	SP 3-PLY	SPE 2-PLY	SPE 3-PLY	SP 2-PLY	SP 3-PLY	SPE 2-PLY	SPE 3-PLY	SP 2-PLY	SP 3-PLY	SPE 2-PLY	SPE 3-PLY	SP 2-PLY	SP 3-PLY	SPE 2-PLY	SPE 3-PLY
4	5	5	6	3	4	4	5	3	4	4	5	2	3	3	4	2	3	3	3	3	2	2	2	3
500	7	10	9	12	6	7	7	10	5	7	7	4	6	6	9	4	5	5	5	7	3	2	4	6
1000	11	14	14	19	8	11	11	14	8	11	10	7	9	10	13	6	8	8	10	10	5	6	6	8
1500	14	19	19	25	11	15	14	19	11	14	14	9	12	13	17	8	10	10	13	6	9	8	11	
2000	18	24	23	31	14	19	18	24	13	18	17	11	15	16	22	10	13	13	17	8	11	10	14	
2500	21	29	28	37	17	22	22	29	16	21	21	13	18	19	26	12	16	15	20	10	13	13	17	
3000	25	33	32	43	20	26	25	34	19	25	24	15	21	23	30	14	18	18	23	11	15	15	19	
3500	29	38	37	49	22	30	29	39	21	29	28	18	24	26	34	16	21	20	27	13	17	17	22	
4000	32	43	42	56	25	34	33	43	24	32	31	20	27	29	39	18	23	23	30	15	19	19	25	
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5000	39	52	51	68	31	41	40	53	29	39	38	24	32	36	47	21	29	28	37	18	24	23	31	
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6500	50	67	65	86	39	52	51	68	38	50	49	31	41	45	60	27	36	35	47	23	30	29	39	
7000	54	71	69	93	42	56	54	72	40	54	52	33	44	48	65	29	39	38	50	24	32	31	42	
7500	57	76	74	99	45	60	58	77	43	57	55	35	47	52	69	31	42	40	53	26	35	33	44	
8000	61	81	79	105	48	64	62	82	46	61	59	38	50	55	73	33	44	43	57	28	37	35	47	
8500	64	86	83	111	51	67	65	87	48	64	62	40	53	58	77	35	47	45	60	29	39	38	50	
9000	68	90	88	117	53	71	69	92	51	68	66	42	56	61	82	37	49	48	63	31	41	40	53	
9500	71	95	93	123	56	75	72	97	54	72	69	44	59	65	86	39	52	50	67	32	43	41	56	
P =	P/70/2	2P/70/3	P/54/2	2P/54/3	P/89/2	2P/89/3	P/69/2	2P/69/3	P/93/2	2P/93/3	P/72/2	2P/72/3	P/113/2	2P/113/3	P/77/2	2P/77/3	P/128/2	2P/128/3	P/100/2	2P/100/3	P/154/2	2P/154/3	P/120/2	2P/120/3

(DETAILS ARE NOT TO SCALE)

NAIL TYPE	NAIL CHARACTERISTICS	
	EDGE MIN SPACING	END
8d BOX (0.1130" x2.5")	3/4	1 3/4
10d BOX (0.1280" x3")	7/8	1 5/8
12d BOX (0.1280" x3.25")	7/8	1 5/8
16d BOX (0.1350" x3.5")	7/8	1 5/8
20d BOX (0.1480" x4")	1	1 7/8
8d COMMON (0.1130" x2.5")	7/8	1 5/8
10d COMMON (0.1480" x3.0")	1	1 7/8
12d COMMON (0.1480" x3.25")	1	1 7/8
16d COMMON (0.16200" x3.5")	1	2
0.120"x2.5" GUN	3/4	1 1/2
0.131"x2.5" GUN	7/8	1 5/8
0.120"x3.0" GUN	3/4	1 1/2
0.131" x3.0" GUN	7/8	1 5/8

GENERAL NOTES

- EDGE DISTANCE AND SPACING SPACING BETWEEN STAGGERED ROWS IS 6d. NAILS MAY NOT BE WITHIN EDGE LINE.
- SPACING OF NAILS IN A ROW IS 12d.
- END DISTANCE IS 15d. IN ADDITION TO NOTE #2, NAILS MAY NOT BE WITHIN END DISTANCES FROM END OF THE BOARD.
- WHEN 3-PLIES ARE USED, INSTALL NAILS INTO 2-PLIES WITH 2x THE NAIL SPACING; THEN ADD THIRD PLY WITH 2x NAIL SPACING.
- RECOMMEND 1 ROW FOR 2x4, 2 ROWS FOR 2x6 & 2x8, 3 ROWS FOR 2x10 & 2x12.
- IF TRUSSES ARE SUPPORTED ON BOTH SIDES, DOUBLE THE SPACING AND ALTERNATE HEADS OF NAILS ON OPPOSING SIDES.



MULTI-PLY TRUSS GIRDER PLY CONNECTOR FOR ISOLATED POINT LOADS (NAILS)

REV: 2.1
 ENG: MDV
 CAD: RC
 DATE: 03/16/18

DRAWING NUMBER
 DR-1

